

AORERE STAGE 2 – TOC45

Superlot AO-016

Geotechnical Completion Report

Document Control

AORERE STAGE 2 – SUPERLOT AO-016– GEOTECHNICAL COMPLETION REPORT

DATE	VERSION	DESCRIPTION	PREPARED BY	REVIEWED BY	AUTHORISED BY
March 2023	1.0	Geotechnical Completion Report	Scott Zhang	Andy Huang	Elby Tang

Distribution

Kāinga Ora - Homes and Communities

1 PDF copy

Piritahi

1 PDF copy

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1.0 Introduction

Piritahi was engaged by Kāinga Ora - Homes and Communities to provide geotechnical observations during the construction stage of the Mangere Development Project, Aorere Neighbourhood Stage 2.

Construction works carried out by Piritahi on Superlot AO-016 comprised:

- Removal of the existing buildings;
- Surficial soil strip to remove contaminated soil;
- Removal of existing stormwater and wastewater lot connections;
- Installation of a new stormwater connection at the northern boundary of the superlot; and
- Upgrade of an existing wastewater line across the centre of the superlot.

2.0 Geotechnical Investigation Report

The Geotechnical Investigation Report for this site was provided by Piritahi, "Geotechnical Report for Land Development – Aorere Stage 2 – Superlot AO-016", dated April 2022, version 1.0.

3.0 Site visits

Site visits to observe construction were carried out as summarised in Table 1 below and discussed in this report. Site visit reports are provided in Appendix A – Site Observation Reports.

Table 1: Summary of geotechnical observations on Superlot AO-016

DATE	INSPECTION BY	DESCRIPTION
21 June 2022	Piritahi Geotechnical Engineer	Manhole subgrade inspection
23 February 2023	Piritahi Geotechnical Engineer	Site walkover and subgrade testing

4.0 Earthworks

4.1 Earthworks subgrade

The existing houses within Superlot AO-016 have been removed. An approximately 300 mm thick surficial soil topsoil strip was carried out across the site extent to remove potential soil contamination. The exposed subgrade following the surface strip comprised natural soils.

Our observations of the surficial soil conditions are presented in the following sections.

Natural Soils

The natural soils exposed comprised of Auckland Volcanic Field (AVF) SILT and clayey SILT. Shear vane and Scala penetrometer testing was undertaken on the finished subgrade level in natural soils by Piritahi Geotechnical Engineers.

Shear vane testing on the existing subgrade measured undrained shear strengths ranging between 86 kPa and >223 kPa, with an average shear strength of 172 kPa. The Scala penetrometer testing recorded blow counts ranging from 0.5 to 3.5 blows per 50 mm penetration across the superlot, averaging 2 blows per 50 mm. The shear vane and Scala penetrometer test results indicate the natural soils at subgrade level are generally in line with the Piritahi Geotechnical Investigation Report.

The test location plans and results for the subgrade testing carried out by Piritahi Geotechnical Engineers are provided in Appendix A – Site Observation Reports.

4.2 Temporary granular hardstand

There is an existing granular hardfill hardstand, approximately 0.2 – 0.5m thick, that was placed above the subgrade at the entrance to the north boundary of the superlot. The location of the granular hardstand is shown on the superlot handover plan in Appendix B. The hardfill was previously placed for temporary construction access of heavy machinery and plant. We were unable to inspect the underlying soils before the placement of the hardfill. This hardfill is not engineered for permanent use as building platforms or pavement subgrades. This hardfill remains in place at the time of this report and it is understood that this superlot is to be handed over with the hardfill left on site for future developers to use.

The Geotechnical Engineer for the future development should confirm that the underlying soils are consistent with the design of the proposed works.

5.0 Infrastructure

5.1 Infrastructure removal

Infrastructure works within Superlot AO-016 comprised the removal of the following utilities:

- Removal of 6 stormwater lot connections. The existing stormwater lot connections were generally within the upper 200mm of subgrade and were removed as part of the surficial topsoil strip.
- Removal of 6 wastewater lot connections. The wastewater lot connections were removed and backfilled with cohesive material (silt and clay) to a depth of approximately 0.5m. Shear vane testing was undertaken by the Piritahi site team within the redundant wastewater connection trenches, measuring undrained shear strengths between 172 and 186 kPa, averaging 175 kPa. The shear vane testing results comply with the Piritahi Earthworks Fill ITP requirements.

The test frequency and methodology are shown on the Piritahi Inspection and Test Plan (ITP). The test location plans and results for the earthworks testing carried out by Piritahi's construction team can be found in Appendix C – Piritahi Test Results. Testing carried out by Geotechnics Technicians can be found in Appendix D – Geotechnics Testing. The as-built plan is attached in Appendix B – As Built Plans, and the Piritahi ITP is attached in Appendix E – Piritahi ITPs.

5.2 Infrastructure Installation

Infrastructure works within Superlot AO-016 comprised the installation of the following utilities:

- Installation of a new stormwater manhole (MH-AO-16) at the north boundary of the superlot connecting to the new stormwater line (at 'Saddle 2-12') along Winthrop Way. Backfill of the stormwater trench were undertaken using GAP20 hardfill. Clegg hammer tests on the GAP20 hardfill recorded Clegg Impact Values (CIV) between 31 and 34. The test results achieved the compaction requirement of the Piritahi Stormwater ITP. The new stormwater manhole installed is shown in Appendix B – As Built Plans. The backfill records are provided in Appendix C – Piritahi Test Results.
- Upgrade of an existing wastewater line across the centre of the superlot, including installation of a manhole (WWMH C-1) near the western boundary. The upgraded wastewater line was backfilled with imported GAP65 hardfill from the trench base to a depth of 2.0 m bgl. Clegg hammer testing was undertaken by the Piritahi site team on the GAP 65 hardfill, measuring Clegg Impact Values (CIV) between 30 and 42, averaging CIV 34 across all lifts. From a trench depth of 2.0 m bgl to the ground surface, the removed wastewater line was backfilled with cohesive material (clay and silt). Shear vane testing was undertaken by the Piritahi site team on the cohesive material, measuring undrained shear strengths between 145 kPa and 174 kPa, averaging 153 kPa. Nuclear densometer testing undertaken by Geotechnics recorded air voids values of between 0.0% and 4.9%, averaging 1.8%. The Clegg Impact Value and Nuclear Densometer testing of the wastewater line backfill material comply with the Piritahi Earthworks Fill ITP requirements.

Testing of the subgrade at the manhole foundation level was undertaken by the Piritahi Geotechnical Engineer, recording an undrained shear strength of 20 to 40 kPa. Backfill of the manhole undercut was undertaken using GAP65 hardfill. Clegg hammer tests on the GAP65 hardfill recorded Clegg Impact Values between 30 and 40. The manhole trench was backfilled with cohesive material (silt and clay). Shear vane testing was undertaken by the Piritahi site team on the cohesive material, measuring undrained shear strength between 145 kPa and 171 kPa, averaging 152 kPa. The test results achieved the compaction requirement of the Piritahi Wastewater ITP. The new wastewater manhole installed is shown in Appendix B – As Built Plans. The backfill records are provided in Appendix C – Piritahi Test Results.

6.0 Statement of professional opinion on suitability of land for building construction

I, Elby Tang, of Piritahi, 139 Quay St, Auckland 1010, hereby confirm that:

1. I am a geo-professional as defined in clause 1.2.2 of NZ 4404:2010 and was retained by the Developer as the geo-professional on Stage 2 of the Aorere Development.
2. The extent of the preliminary investigations is described in the Piritahi Geotechnical Investigation Report provided by Piritahi (dated April 2022, version 1.0). As noted in the report, the scope of this investigation report is limited, as it was carried out prior to design of the development. Depending on the proposed development, supplementary geotechnical investigations, analysis and design may be required to inform detailed design and building consent.
3. Construction works carried out by Piritahi at this site comprise:
 - a. Removal of existing buildings;
 - b. Stripping of surficial soils to remove potential contaminants;
 - c. Decommissioning and removal of residential stormwater and wastewater lot connections (within the surficial topsoil strip);
 - d. Installation of a new stormwater connection; and
 - e. Upgrade of an existing wastewater line.
4. The extent of inspections during construction, and the results of all the tests and/or evaluations carried out are described in this Piritahi Geotechnical Completion Report dated March 2023, version 1.0.
5. In my professional opinion, not to be construed as a guarantee, I consider that:
 - a. Observations of the underlying soils by Piritahi to date are generally in line with the findings of the Piritahi Geotechnical Investigation Report.
 - b. As noted in the Piritahi Geotechnical Investigation Report for AO-016, the undrained shear strength of the natural soils meet the definition of "good ground" as outlined in NZS 3604-2011. However, the natural soils at the site do not meet "good ground" criteria for soil expansivity and liquefaction susceptibility, and foundations on such soils will require consideration of the potential effects of shrink-swell and liquefaction on the development.
 - c. The temporary granular hardstand remain at the northern entrances to the site. This hardstand is not engineered for permanent use as building platforms or pavement subgrades and should be remediated or removed prior to future developments.
 - d. No backfill was undertaken following the removal of the stormwater lot connections that were generally 200 mm deep (within the topsoil strip).
 - e. The removed wastewater lot connections were backfilled with compacted cohesive material to a depth of about 0.5 m. Testing of this cohesive material achieved shear vane results greater than 175 kPa. The cohesive backfill material complies with the compaction requirements of the Piritahi Earthworks Fill ITP.
 - f. A new stormwater connection, installed at the northern boundary of the site was backfilled with GAP20 hardfill. This backfill achieved Clegg impact values of greater than and complies with the Piritahi Stormwater ITP.

- g. The upgraded public wastewater line was backfilled with compacted GAP65 hardfill from the trench subgrade to a depth of approximately 2.0 m bgl. Testing of this GAP65 hardfill achieved Clegg impact values greater than 30. From 2.0 m bgl to the finished subgrade level, the trench was backfilled with cohesive material (clay and silt). Testing of this cohesive material achieved undrained shear strength greater than 130 kPa and air voids less than 8%. The GAP65 hardfill and cohesive material comply with the compaction requirements of the Piritahi Wastewater ITP.
 - h. A new wastewater manhole, installed at the western boundary of the site was backfilled with GAP65 hardfill from trench undercut level to trench subgrade level, and with cohesive material (silt and clay) from trench subgrade level to the finished ground subgrade level. The GAP65 backfill achieved Clegg impact values of greater than 30 and the cohesive backfill material achieved undrained shear strength of greater than 130 kPa. Backfill materials comply with the Piritahi Wastewater ITP.
 - i. The subgrade may deteriorate if left exposed to the weather for an extended period or to construction traffic. For this reason, the soils should be reinspected by a Chartered Geotechnical Engineer at the time of construction of any building foundations or pavements. It may be necessary to remove or recompact soils that have lost strength due to exposure to weather or traffic.
 - j. The design of future works should consider the upgraded wastewater line across the centre and the new stormwater line at the northern boundary of the site, as shown on the as built plan (Appendix B). This may require avoidance, bridging or piling around the pipes.
6. This professional opinion is furnished to the territorial authority and the developer for their purposes alone on the express condition that it will not be relied upon by any other person and does not remove the necessity for the normal inspection of foundation conditions at the time of erection of any building.
 7. This certificate shall be read in conjunction with my geotechnical report referred to in clause 2 above and shall not be copied or reproduced except in conjunction with the full geotechnical completion report and the geotechnical investigation report.
 8. General guidance for homeowners regarding expansive soils has been enclosed in Appendix F – Foundation maintenance and footing performance: a homeowner’s guide of this report.

Signed:



Date: 24 March 2023

Elby Tang

BE Civil (Hons), ME Civil (Hons), CMEngNZ

CPEng 1020514

AC Author PSA125042

7.0 Applicability

This report has been prepared for the exclusive use of our client Kāinga Ora - Homes and Communities Ltd with respect to the particular brief given to us and it may not be relied upon in other contexts or for any other purpose, or by any person other than our client, without our prior written agreement.

Appendix A – Site Observation Reports

SITE VISIT RECORD SHEET

Project Name: Aorere Stage 2	Project No.: TOC045
Ref #: WWMH C-1	Date/Time: 21/06/2022
Site Location: AO-016	Inspection by: NIBU
Weather: Fine	Inspection with: Amrit Singh
Site Condition: Good	Purpose of Visit: MH subgrade observation and testing
Site Induction Completed? <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No	JSEA completed? <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No
<p>Health and safety observation notes:</p> <ul style="list-style-type: none"> - Working at heights - Working around moving plant - S/T/F - Collapse of trench 	
<p>Machinery onsite:</p> <ul style="list-style-type: none"> - 1 20t excavator 	
<p>Dust/erosion & sediment control:</p> <ul style="list-style-type: none"> - NA 	
<p>Current active works:</p> <ul style="list-style-type: none"> - Excavation and installation of the WW line through AO-016 and AO-015 	
<p>Observations:</p> <p>The construction had completed the excavation for WWMH C-1, as named on the Piritahi Wastewater Layout Plan Overall (drawing number TOC045-002-5000). The excavation extended to between 3.0 m and 3.5 m from the surrounding ground surface (natural ground surface following the topsoil strip). The material within the trench consisted of Auckland Volcanic Field silts and clays extending to 2.5 m to 2.8 m from ground surface which was underlain by sandy, Alluvial clay and silt. The area had been benched by the construction team and the AVF material appeared stable.</p> <p>Shear vane testing was carried out on the alluvial material at the proposed subgrade level for the manhole. This level was based on the depth surveyed by the construction team at approximately 3.0 m from the ground level. Corrected shear strength results ranged from 20 kPa – 40 kPa within the alluvium suggesting an undercut of 500 mm would be required as per the wastewater ITP. The construction team carried out a test undercut of 500 mm and exposed further alluvial material of a similar shear strength to that recorded at subgrade level. Further undercut of that material was not recommended and only the undercut requirements laid out in the ITP should be followed.</p>	
<p>Agreements/recommendations onsite:</p> <ul style="list-style-type: none"> - Recommended, based on the shear vane results, an undercut of 500 mm as per the wastewater ITP. - Recommended carrying out ongoing observations of the soil beneath the excavator as a health and safety precaution as it looked a little unstable. 	
<p>Follow-ups and further actions required:</p> <ul style="list-style-type: none"> - Follow up with construction team on visit for next manhole WWMH C-2 - Follow up on backfill methodology. - Follow up on QA testing following backfill. 	

SITE VISIT RECORD SHEET

Sketches/Additional comments:



Photograph 1: Overview of excavation for WWMH C-1



Photograph 2: Transition from AVF to Alluvium.

SITE VISIT RECORD SHEET



Photograph 3: Shear vane testing in the Alluvium at the manhole subgrade level.

SITE VISIT RECORD SHEET

Project Name: Aorere Stage 2	Project ID: 1007708.2086
Ref #: DIM086	Date/Time: 23/02/2023
Site Location: Superlot AO-016 (Winthrop Way, Mangere East)	Inspection by: Scott Zhang and Cameron Taylor
Weather: Fine	Inspection with: N/A
Site Condition: Dry	Purpose of Visit: Site walkover and subgrade testing
Site Induction Completed? <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No	JSEA completed? <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No
Health and safety observation notes: Be aware of slips/trips/falls hazards	
Machinery onsite: 2x7T excavators. 1x20T excavator	
Dust/erosion & sediment control: N/A	
Current active works: N/A	
Observations: <p>Two representatives of the Piritahi geotechnical team undertook a site walkover and subgrade testing for the Superlot AO-016 within TOC45. Cohesive fill (clayey SILT/silty CLAY) were observed at the subgrade cut level (beneath the grass/vegetation) across the superlot.</p> <p>Shear vane and Scala penetrometer testing were conducted across the subgrade.</p> <p>Shear vane testing was undertaken in fine grained soils at 28 locations along with Scala penetrometer tests to 900 mm depth. Test locations are shown in the attached site testing plan.</p> <p>Corrected peak undrained shear strengths of between 86 and 223+ kPa were recorded, for an average of 172 kPa. This indicates the surficial soils at subgrade level is typically stiff to hard. Scala penetrometer blow counts generally ranged from 0.5 to 3.5 blows per 50 mm penetration on the subgrade.</p> <p>The results for the subgrade tests and a site testing location plan are attached. These results indicate the subgrade soils are within the ITP requirements for earthworks cut.</p>	

SITE VISIT RECORD SHEET

Agreements/recommendations onsite:

Nil.

Follow-ups and further actions required:

Piritahi geotechnical team to prepare Geotechnical Completion Report for Superlot AO-016.

Testing results and Site plan attached. Photos below.



Subgrade Testing Plan

SITE VISIT RECORD SHEET



Photograph 1: Superlot overview – looking west



Photograph 2: Superlot overview – looking south

SITE VISIT RECORD SHEET



Photograph 3: Superlot overview – looking north



Photograph 4: Hardstand near the northern boundary (entrance) of the superlot

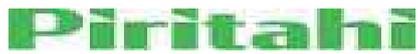


Shear Vane Results

Job No: 1007708.2086	Date: 23/02/2023	Test No.
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	
		Sheet of 1 of 29

Test	Corrected Undrained Shear Strength
1	162
2	223
3	UTP
4	UTP
5	219
6	162
7	223
8	223
9	156
10	162
11	140
12	162
13	188
14	172
15	UTP
16	UTP
17	153

Test	Corrected Undrained Shear Strength
18	UTP
19	194
20	137
21	159
22	146
23	99
24	114
25	89
26	86
27	162
28	178

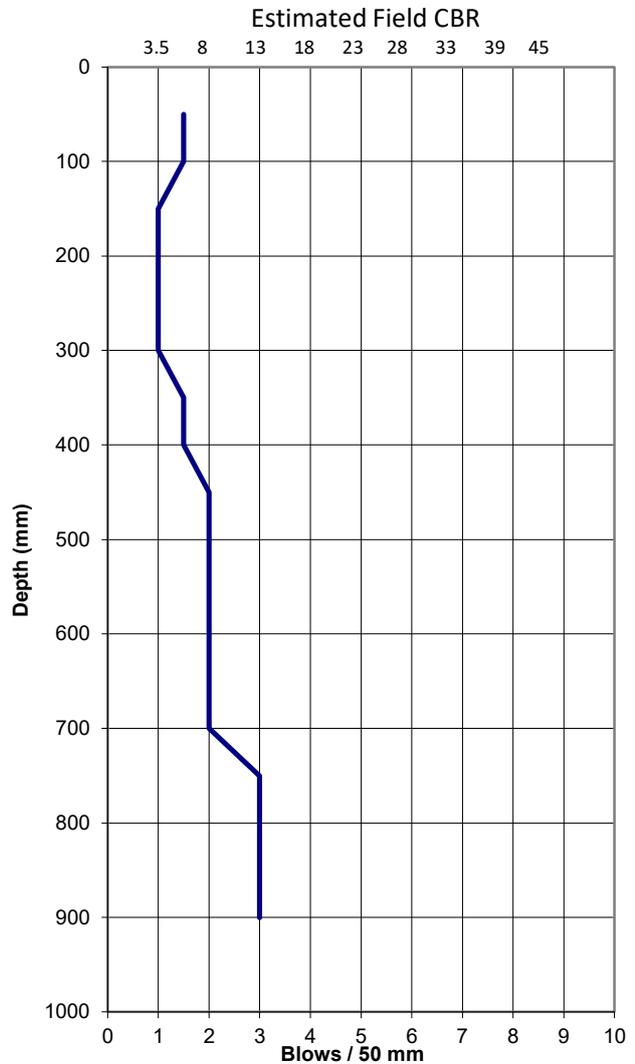


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SCALA PENETROMETER LOG

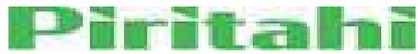
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP01 Sheet 2 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1.5
100	1.5
150	1
200	1
250	1
300	1
350	1.5
400	1.5
450	2
500	2
550	2
600	2
650	2
700	2
750	3
800	3
850	3
900	3



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTRROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

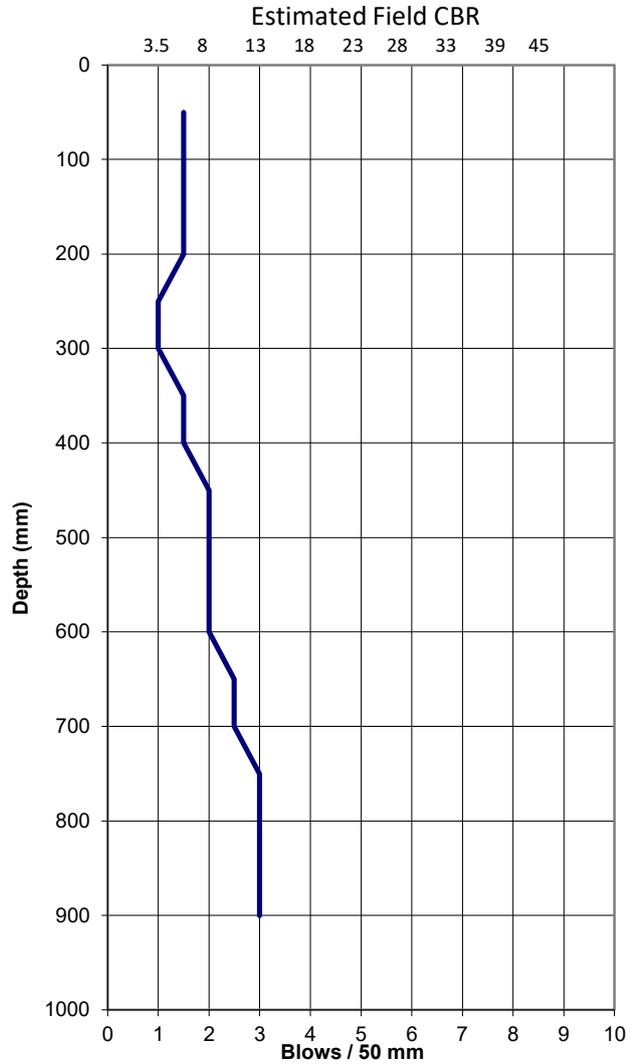


Piritahi

SCALA PENETROMETER LOG

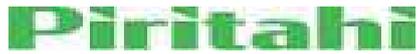
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP02 Sheet 3 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1.5
100	1.5
150	1.5
200	1.5
250	1
300	1
350	1.5
400	1.5
450	2
500	2
550	2
600	2
650	2.5
700	2.5
750	3
800	3
850	3
900	3



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

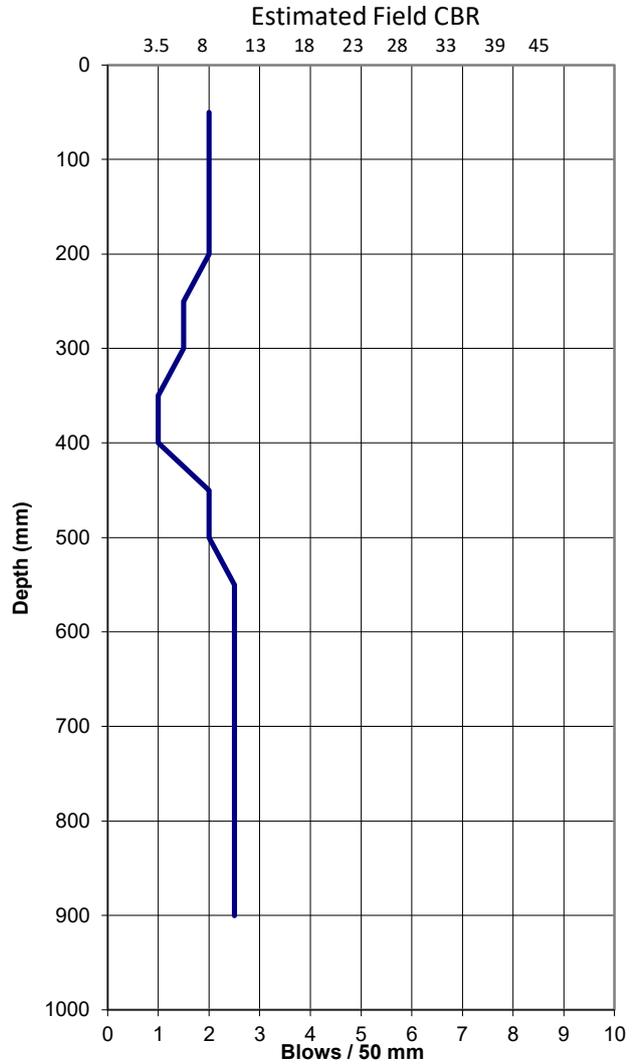


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP03 Sheet 4 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2
100	2
150	2
200	2
250	1.5
300	1.5
350	1
400	1
450	2
500	2
550	2.5
600	2.5
650	2.5
700	2.5
750	2.5
800	2.5
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

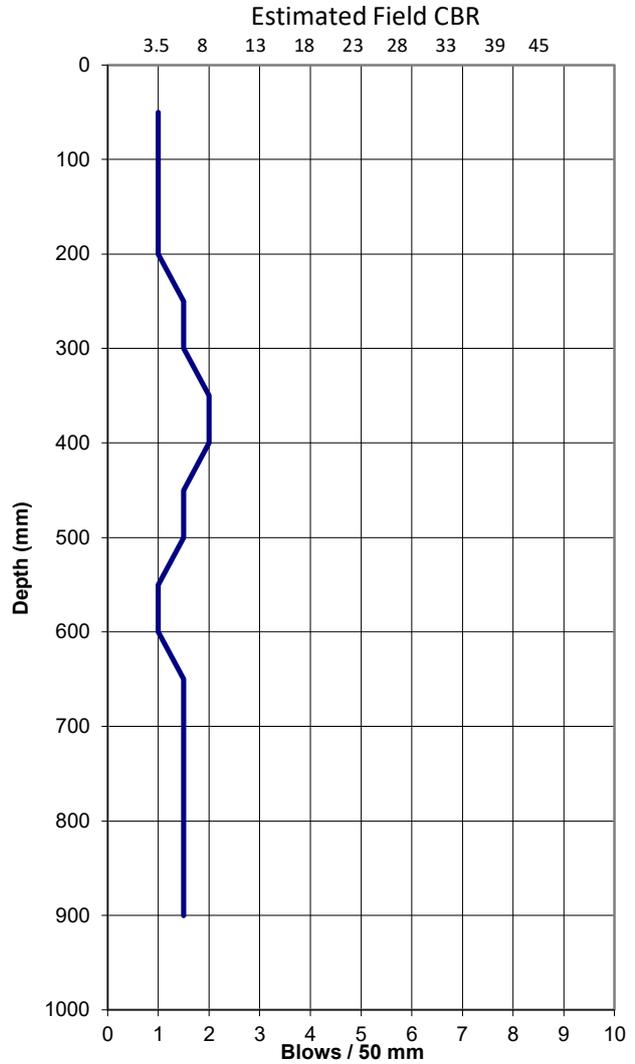


Piritahi

SCALA PENETROMETER LOG

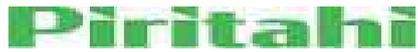
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Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1
100	1
150	1
200	1
250	1.5
300	1.5
350	2
400	2
450	1.5
500	1.5
550	1
600	1
650	1.5
700	1.5
750	1.5
800	1.5
850	1.5
900	1.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTRROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

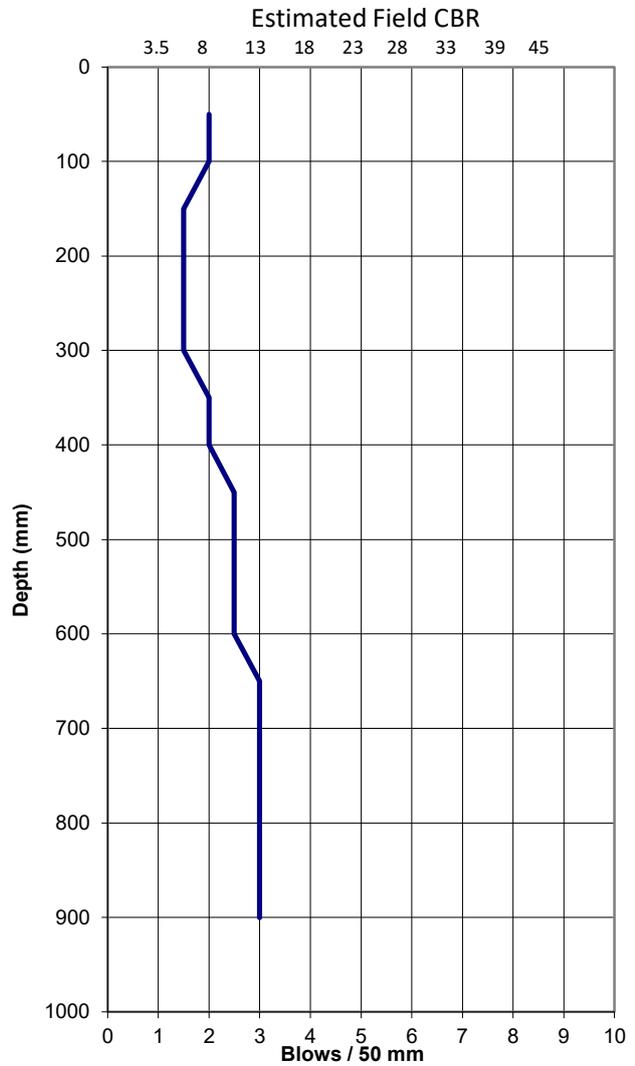


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SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP05 Sheet 6 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2
100	2
150	1.5
200	1.5
250	1.5
300	1.5
350	2
400	2
450	2.5
500	2.5
550	2.5
600	2.5
650	3
700	3
750	3
800	3
850	3
900	3



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

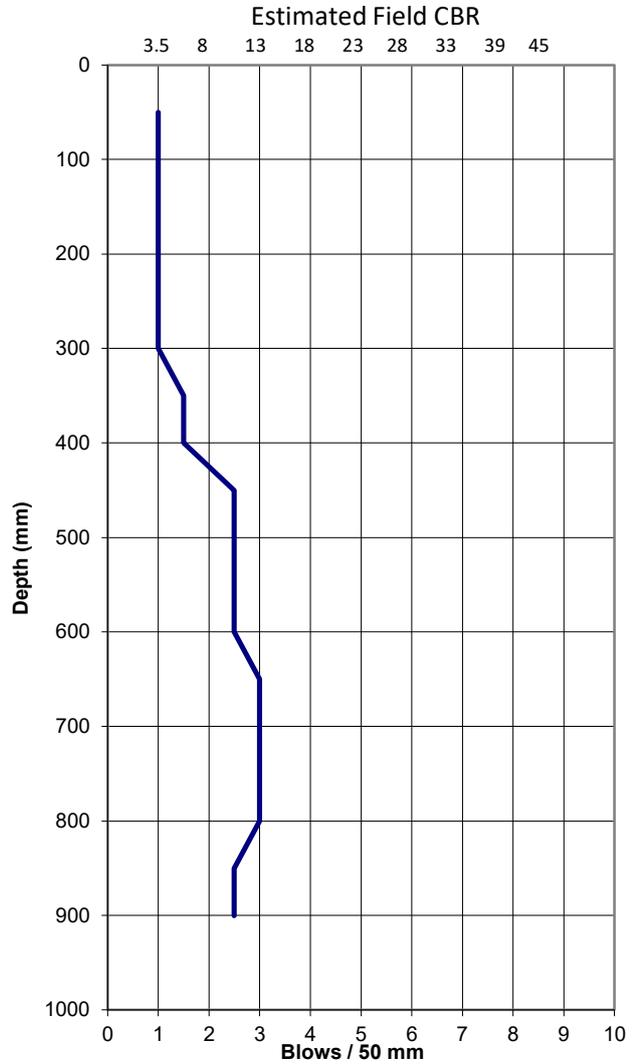


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SCALA PENETROMETER LOG

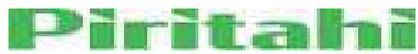
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP06 Sheet 7 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1
100	1
150	1
200	1
250	1
300	1
350	1.5
400	1.5
450	2.5
500	2.5
550	2.5
600	2.5
650	3
700	3
750	3
800	3
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

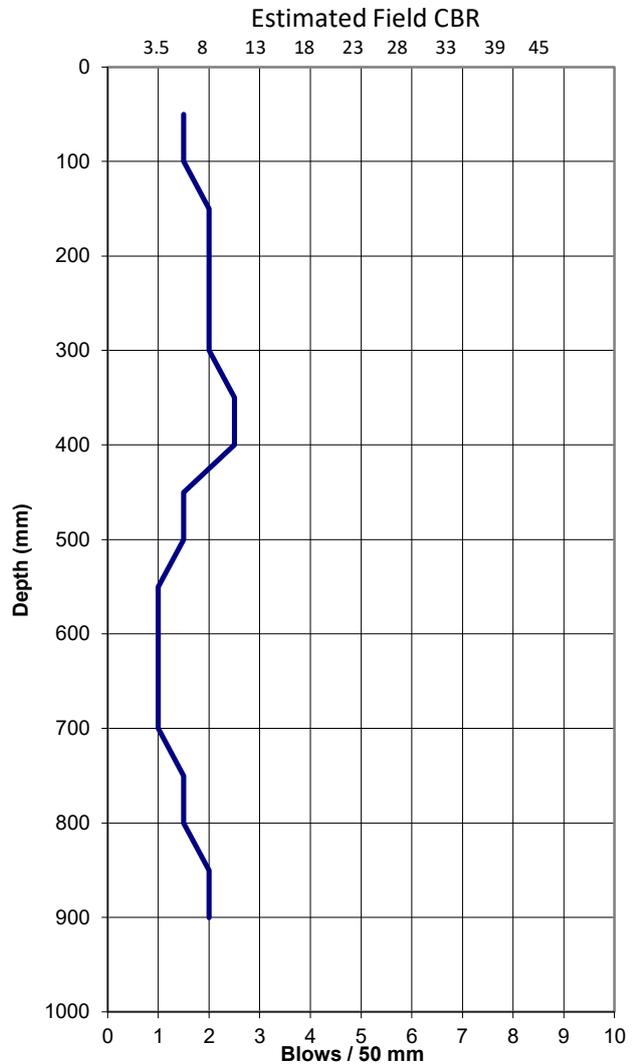


Piritahi

SCALA PENETROMETER LOG

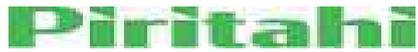
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP07 Sheet 8 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1.5
100	1.5
150	2
200	2
250	2
300	2
350	2.5
400	2.5
450	1.5
500	1.5
550	1
600	1
650	1
700	1
750	1.5
800	1.5
850	2
900	2



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

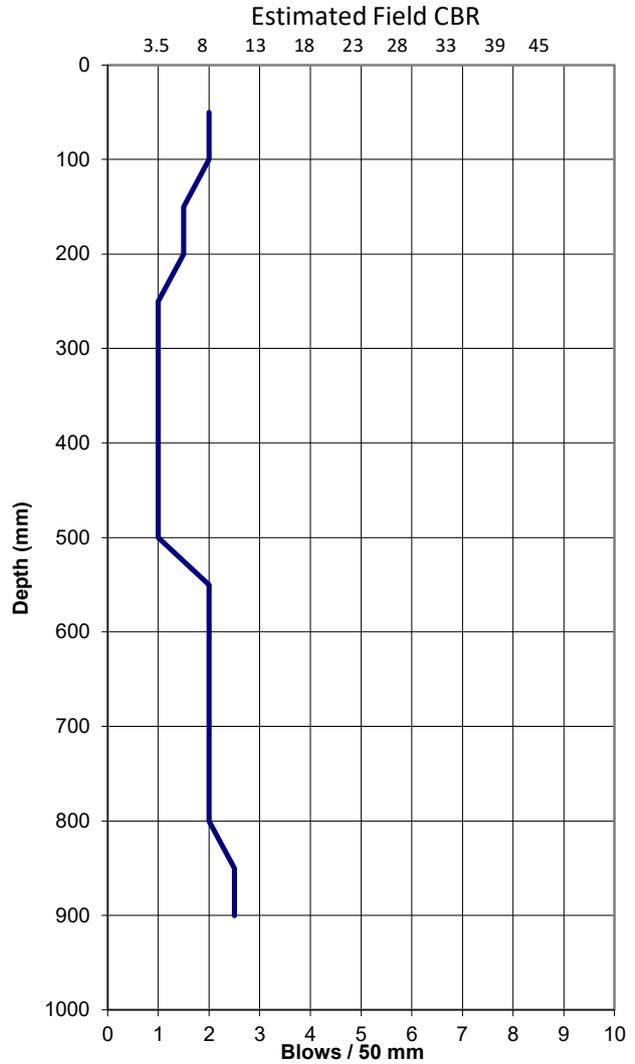


Piritahi

SCALA PENETROMETER LOG

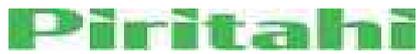
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP08 Sheet 9 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2
100	2
150	1.5
200	1.5
250	1
300	1
350	1
400	1
450	1
500	1
550	2
600	2
650	2
700	2
750	2
800	2
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

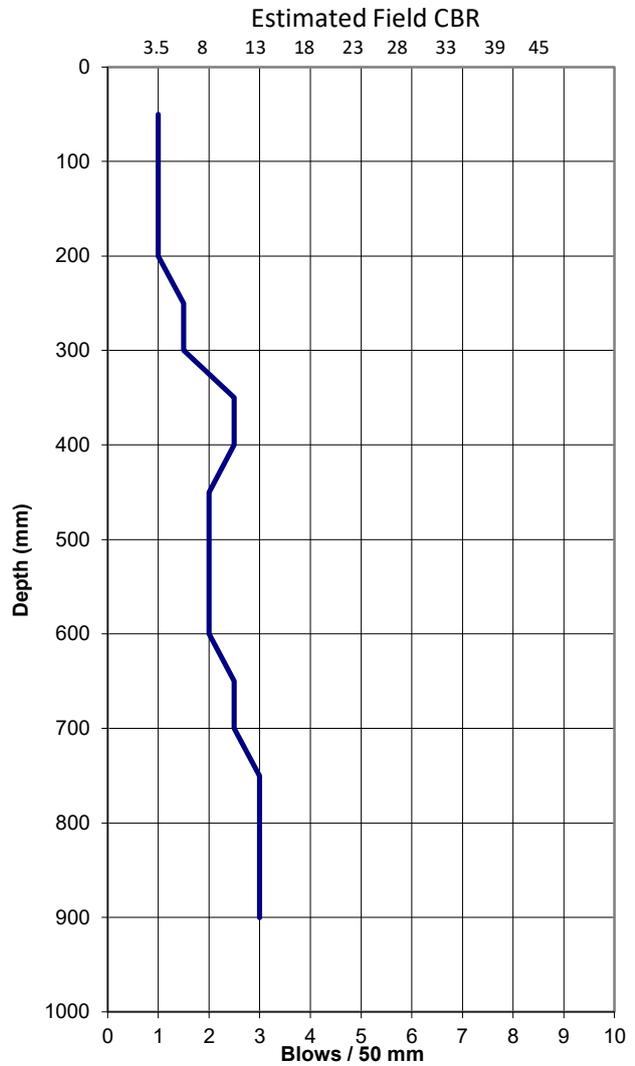


Piritahi

SCALA PENETROMETER LOG

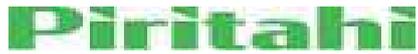
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP09
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1
100	1
150	1
200	1
250	1.5
300	1.5
350	2.5
400	2.5
450	2
500	2
550	2
600	2
650	2.5
700	2.5
750	3
800	3
850	3
900	3



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

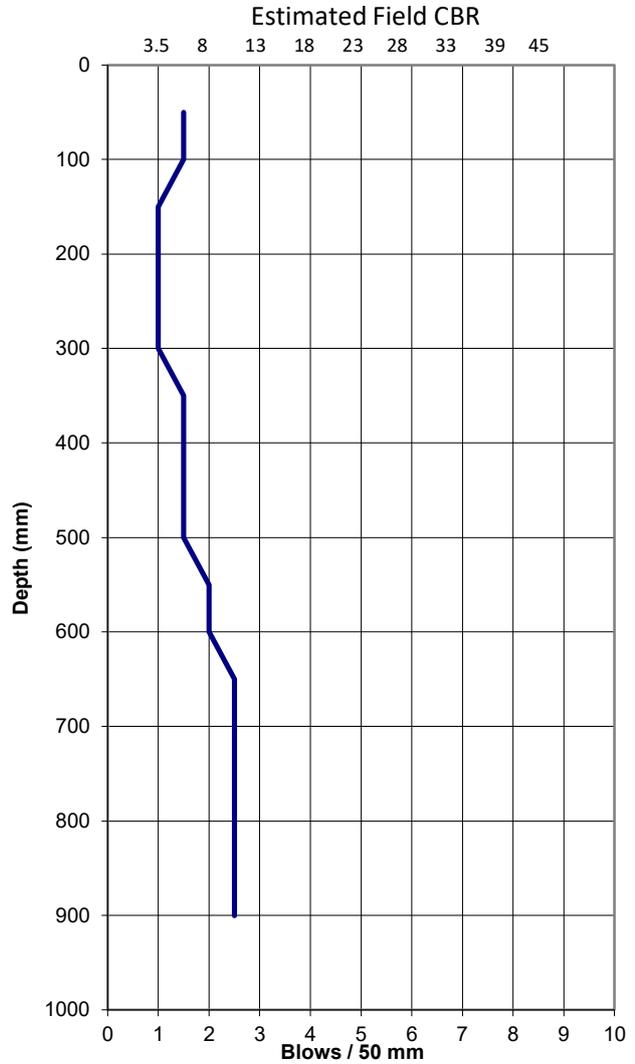


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP10 Sheet 11 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1.5
100	1.5
150	1
200	1
250	1
300	1
350	1.5
400	1.5
450	1.5
500	1.5
550	2
600	2
650	2.5
700	2.5
750	2.5
800	2.5
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

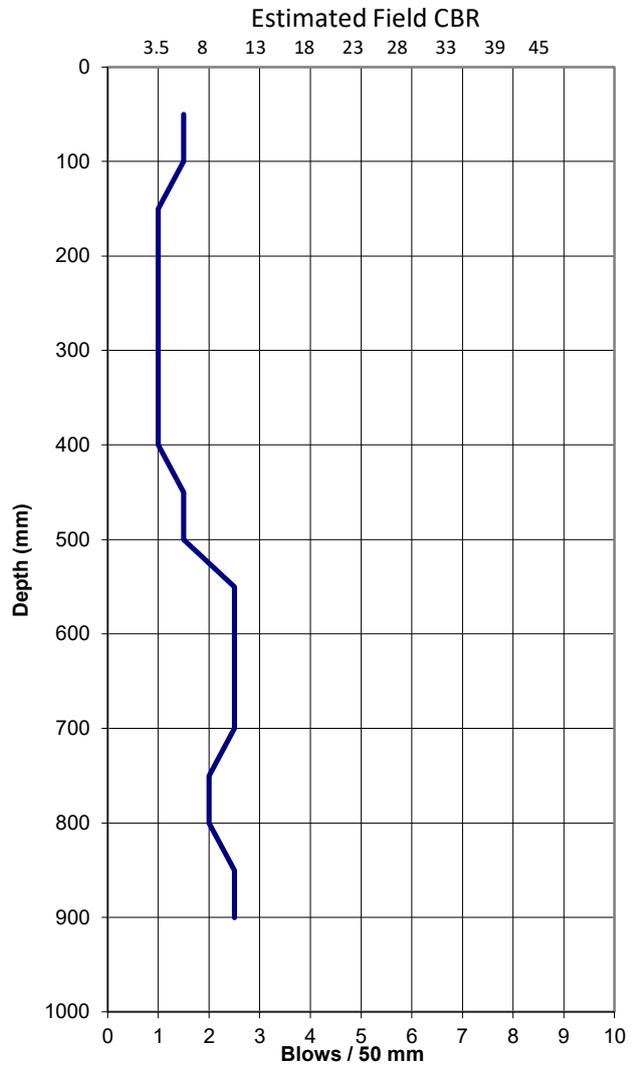


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP11 Sheet 12 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1.5
100	1.5
150	1
200	1
250	1
300	1
350	1
400	1
450	1.5
500	1.5
550	2.5
600	2.5
650	2.5
700	2.5
750	2
800	2
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

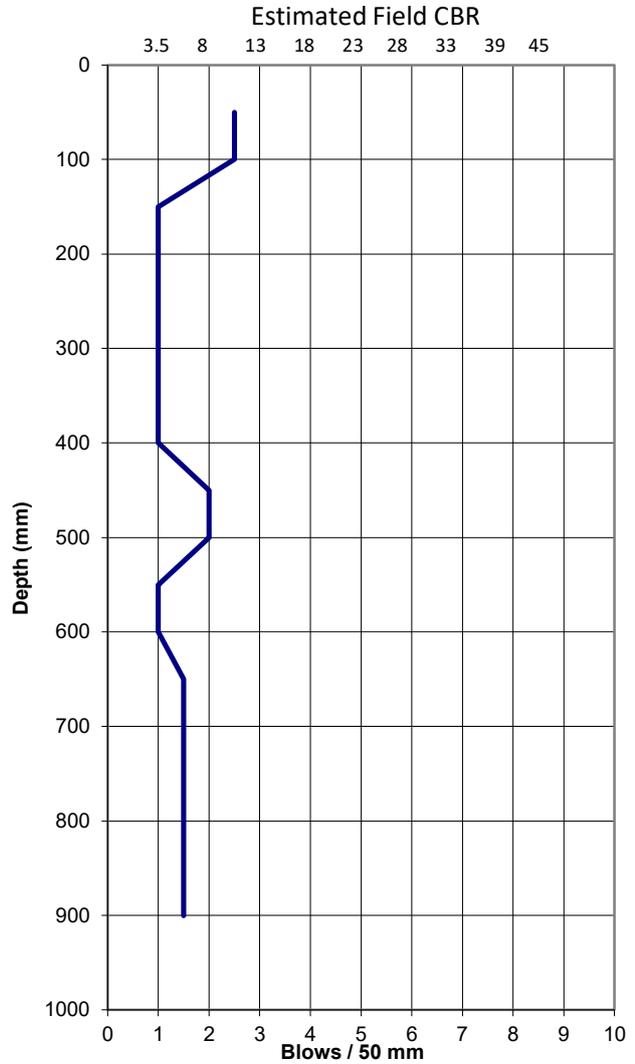


Piritahi

SCALA PENETROMETER LOG

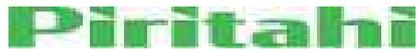
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP12
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2.5
100	2.5
150	1
200	1
250	1
300	1
350	1
400	1
450	2
500	2
550	1
600	1
650	1.5
700	1.5
750	1.5
800	1.5
850	1.5
900	1.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

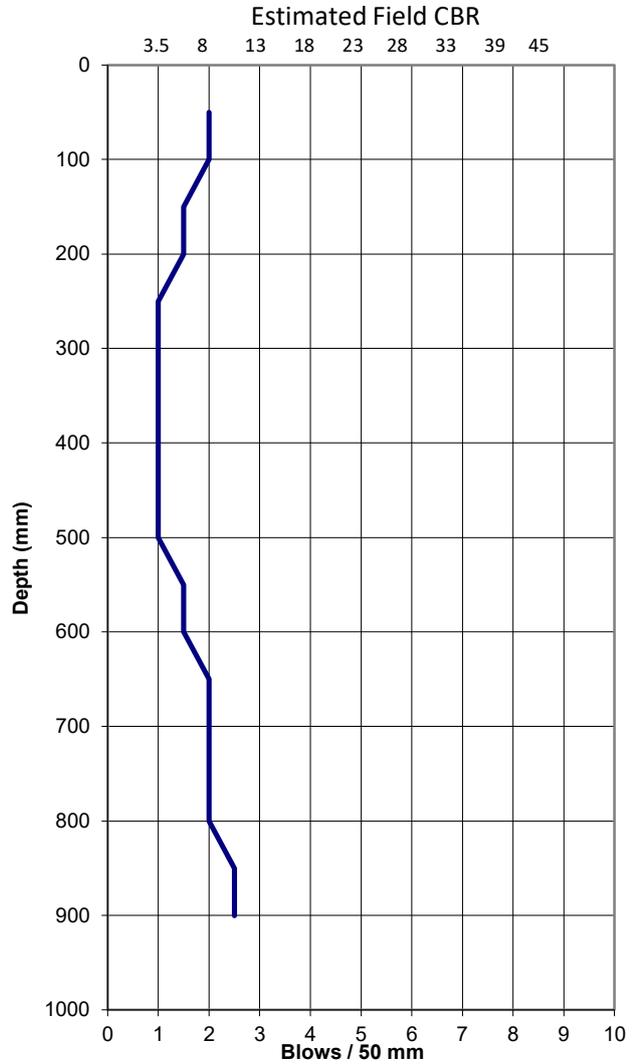


Piritahi

SCALA PENETROMETER LOG

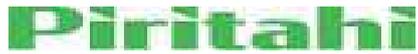
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP13 Sheet 14 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2
100	2
150	1.5
200	1.5
250	1
300	1
350	1
400	1
450	1
500	1
550	1.5
600	1.5
650	2
700	2
750	2
800	2
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

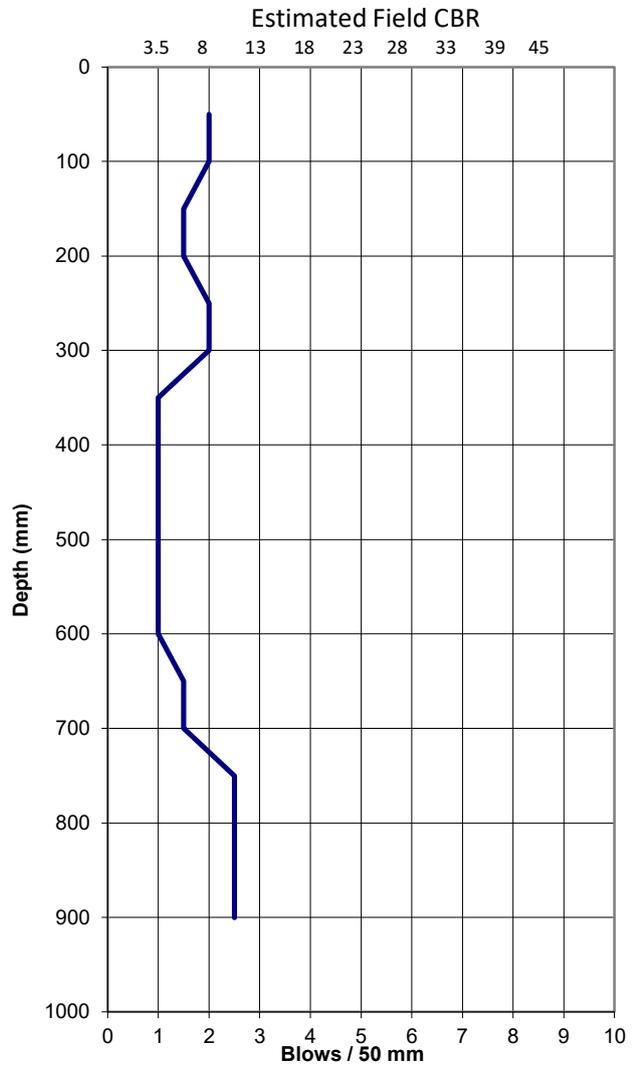


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP14 Sheet 15 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2
100	2
150	1.5
200	1.5
250	2
300	2
350	1
400	1
450	1
500	1
550	1
600	1
650	1.5
700	1.5
750	2.5
800	2.5
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

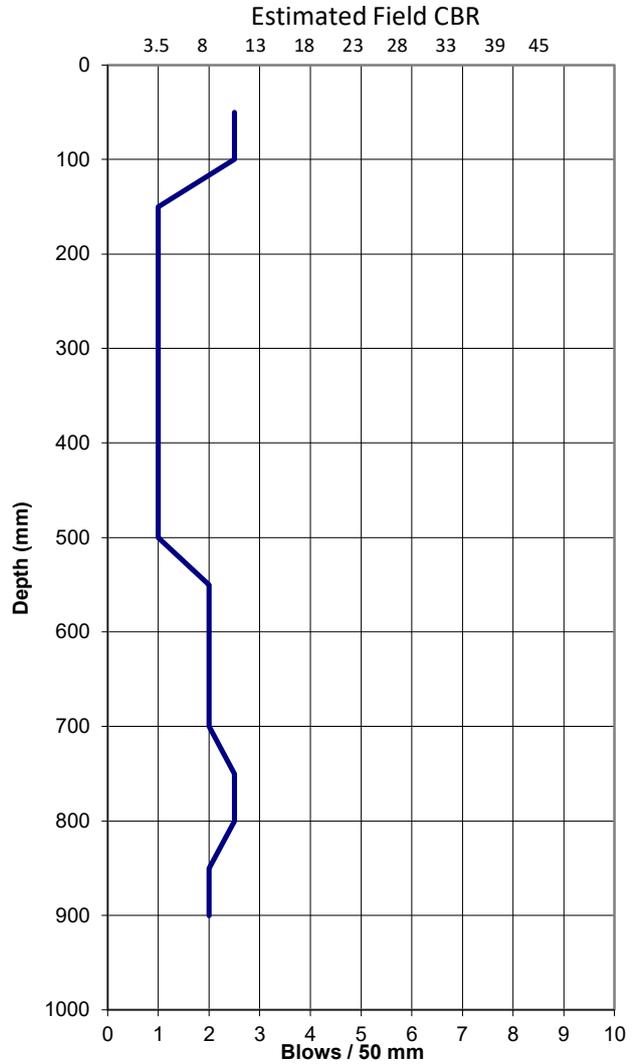


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP15
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2.5
100	2.5
150	1
200	1
250	1
300	1
350	1
400	1
450	1
500	1
550	2
600	2
650	2
700	2
750	2.5
800	2.5
850	2
900	2



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

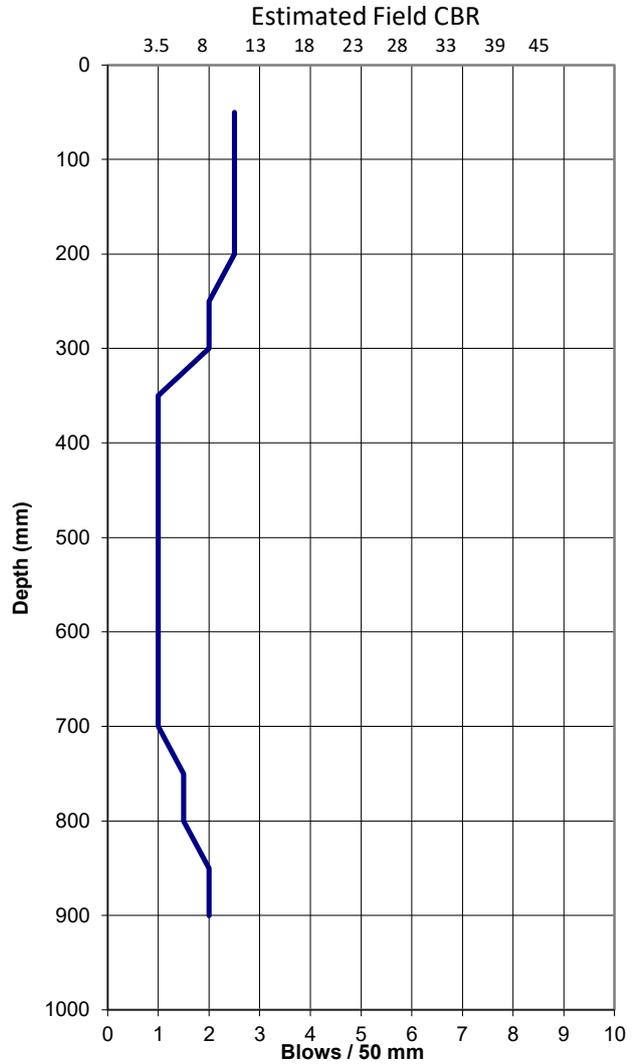


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP16
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2.5
100	2.5
150	2.5
200	2.5
250	2
300	2
350	1
400	1
450	1
500	1
550	1
600	1
650	1
700	1
750	1.5
800	1.5
850	2
900	2



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

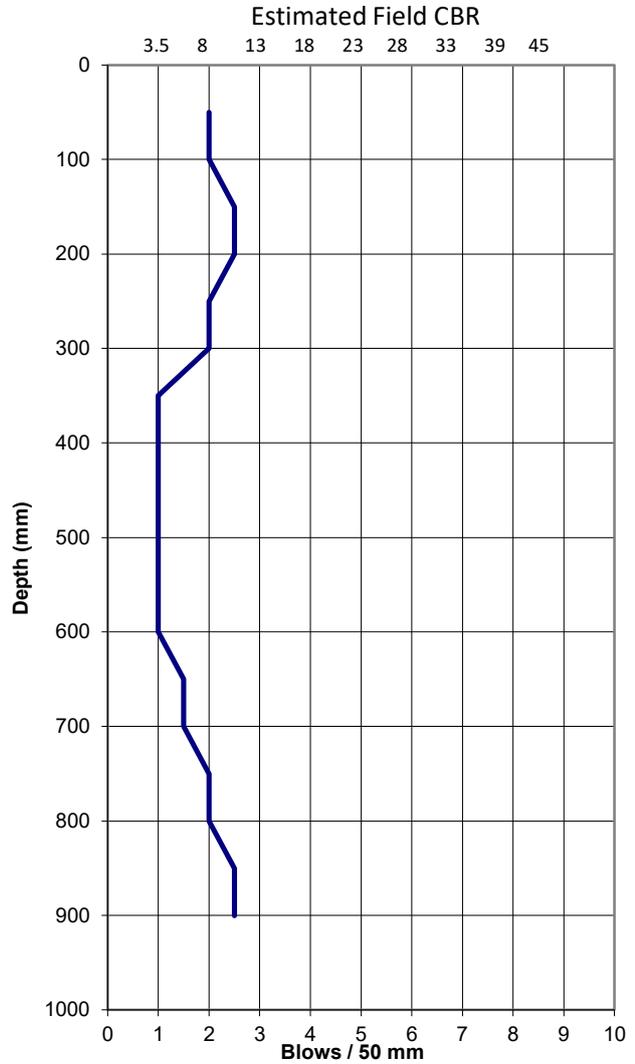


Piritahi

SCALA PENETROMETER LOG

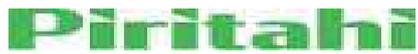
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP17
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2
100	2
150	2.5
200	2.5
250	2
300	2
350	1
400	1
450	1
500	1
550	1
600	1
650	1.5
700	1.5
750	2
800	2
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

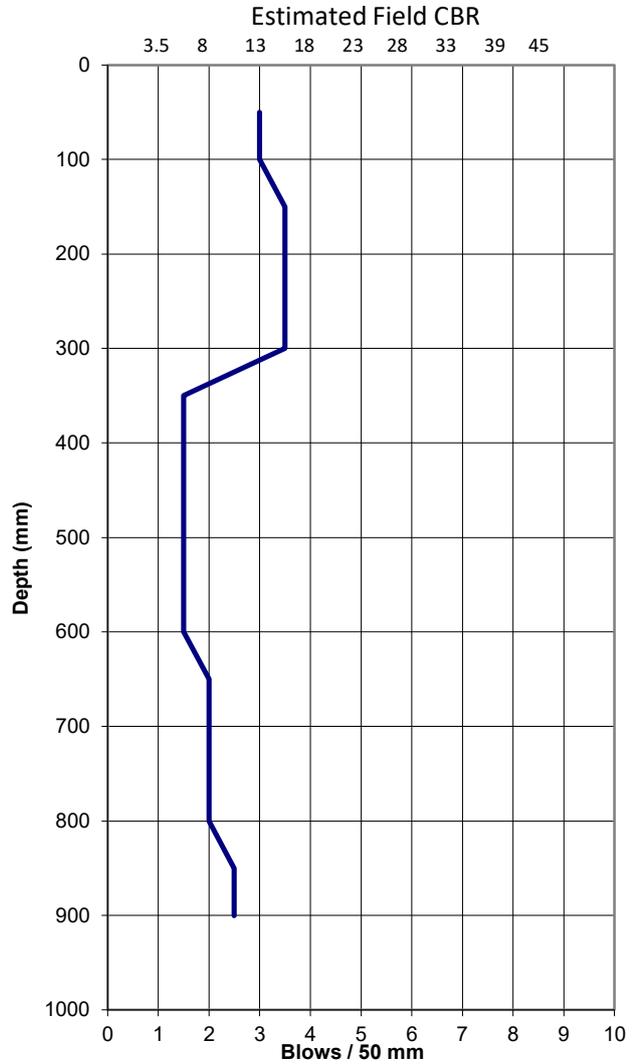


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP18 Sheet 19 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	3
100	3
150	3.5
200	3.5
250	3.5
300	3.5
350	1.5
400	1.5
450	1.5
500	1.5
550	1.5
600	1.5
650	2
700	2
750	2
800	2
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

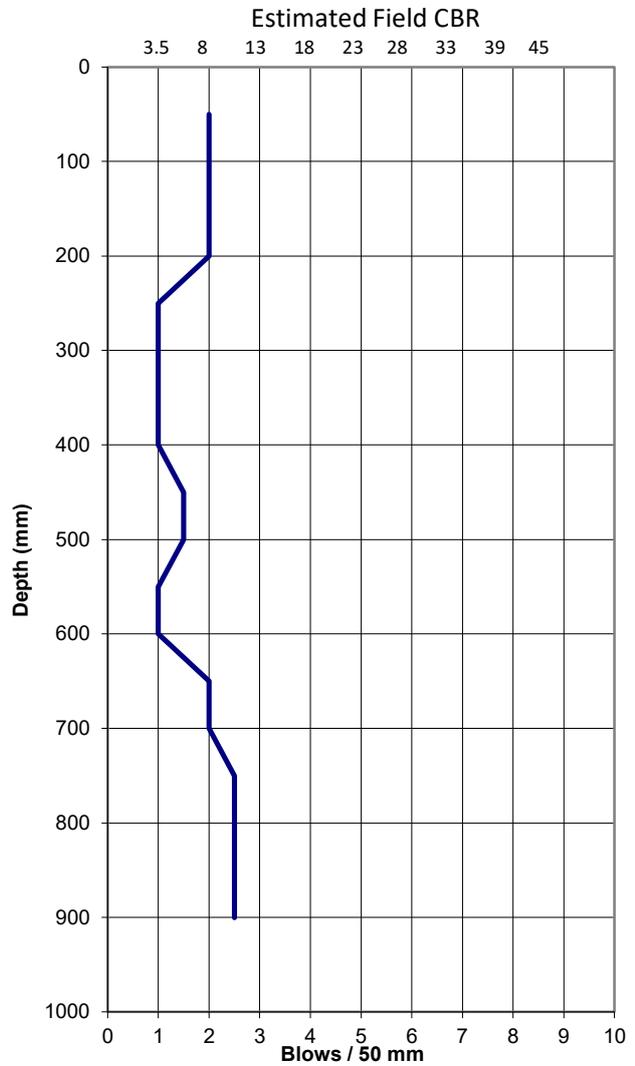


Piritahi

SCALA PENETROMETER LOG

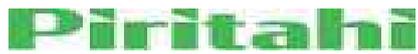
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP19 Sheet 20 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2
100	2
150	2
200	2
250	1
300	1
350	1
400	1
450	1.5
500	1.5
550	1
600	1
650	2
700	2
750	2.5
800	2.5
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

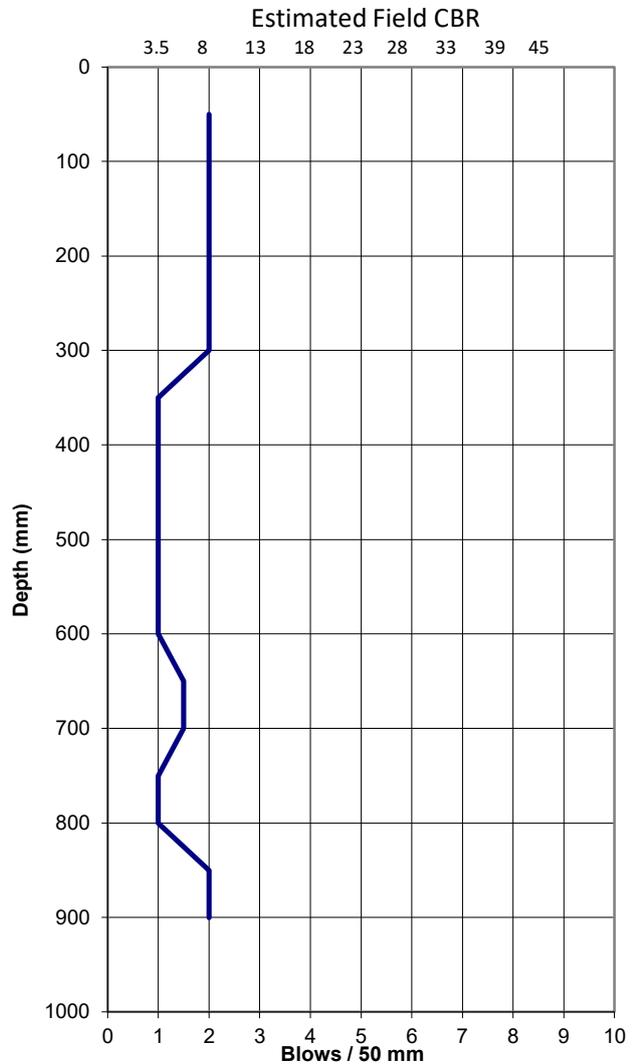


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP20 Sheet 21 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2
100	2
150	2
200	2
250	2
300	2
350	1
400	1
450	1
500	1
550	1
600	1
650	1.5
700	1.5
750	1
800	1
850	2
900	2



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

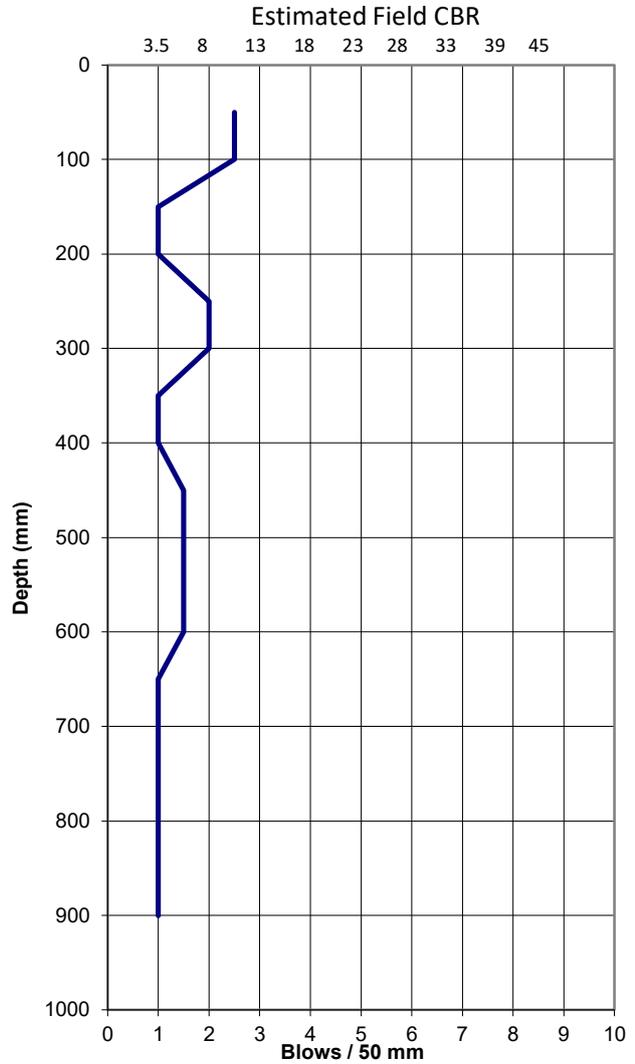


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP21 Sheet 22 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2.5
100	2.5
150	1
200	1
250	2
300	2
350	1
400	1
450	1.5
500	1.5
550	1.5
600	1.5
650	1
700	1
750	1
800	1
850	1
900	1



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

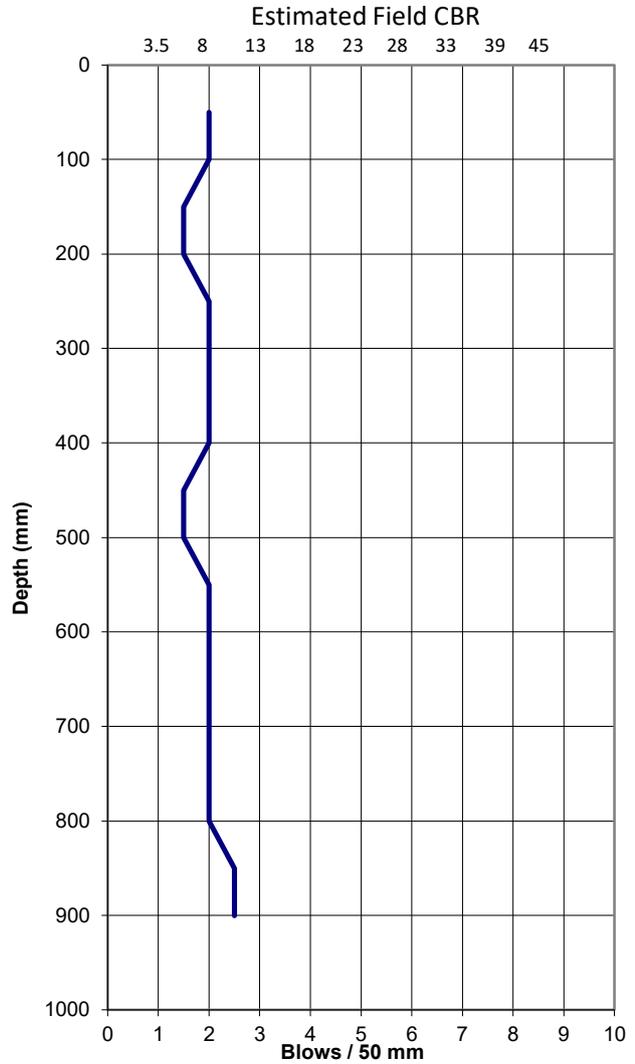


Piritahi

SCALA PENETROMETER LOG

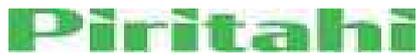
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP22 Sheet 23 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2
100	2
150	1.5
200	1.5
250	2
300	2
350	2
400	2
450	1.5
500	1.5
550	2
600	2
650	2
700	2
750	2
800	2
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTRROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

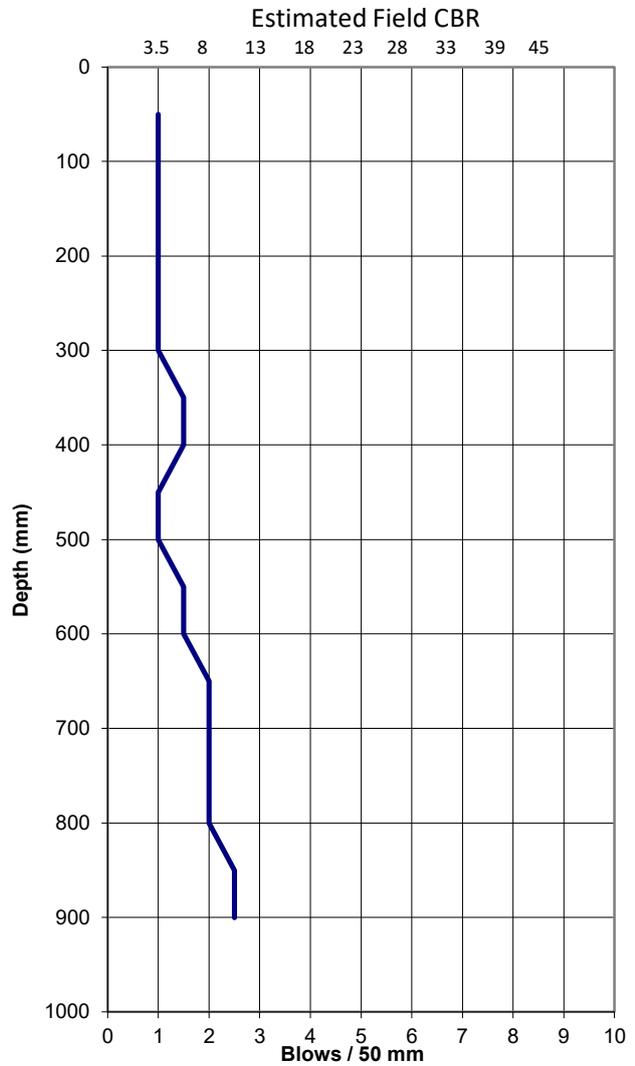


Piritahi

SCALA PENETROMETER LOG

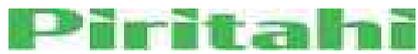
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP23 Sheet 24 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1
100	1
150	1
200	1
250	1
300	1
350	1.5
400	1.5
450	1
500	1
550	1.5
600	1.5
650	2
700	2
750	2
800	2
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTRROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

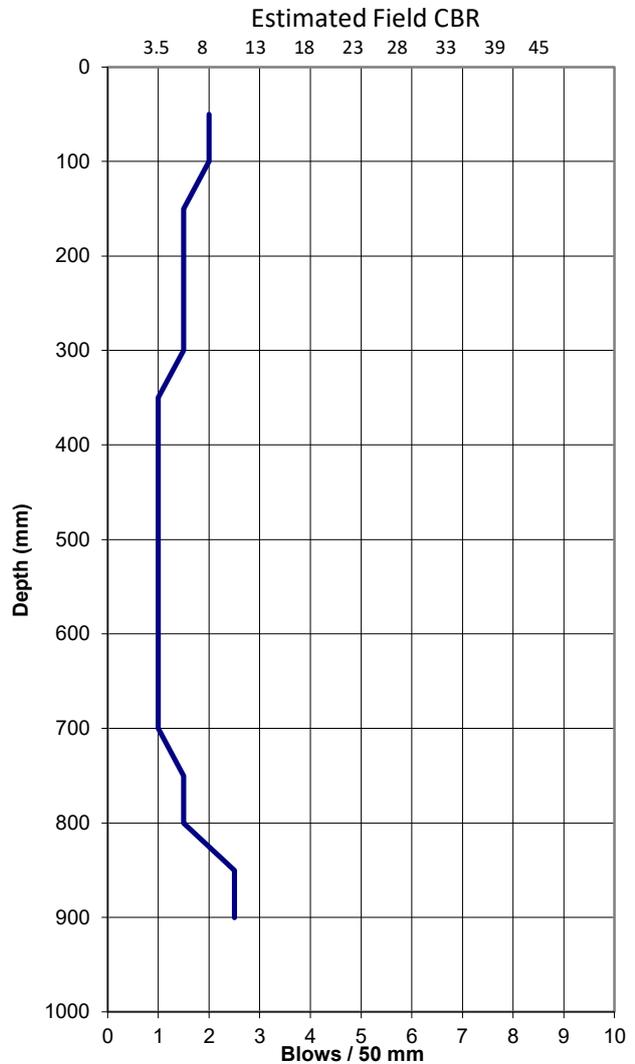


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP23 Sheet 25 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2
100	2
150	1.5
200	1.5
250	1.5
300	1.5
350	1
400	1
450	1
500	1
550	1
600	1
650	1
700	1
750	1.5
800	1.5
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTRROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

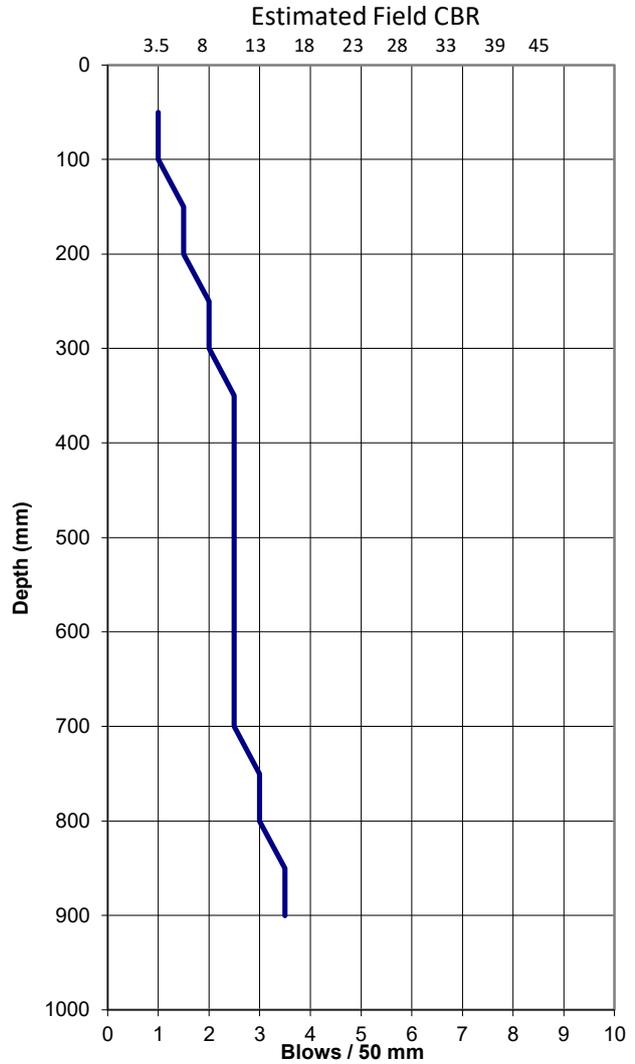


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP25
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1
100	1
150	1.5
200	1.5
250	2
300	2
350	2.5
400	2.5
450	2.5
500	2.5
550	2.5
600	2.5
650	2.5
700	2.5
750	3
800	3
850	3.5
900	3.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

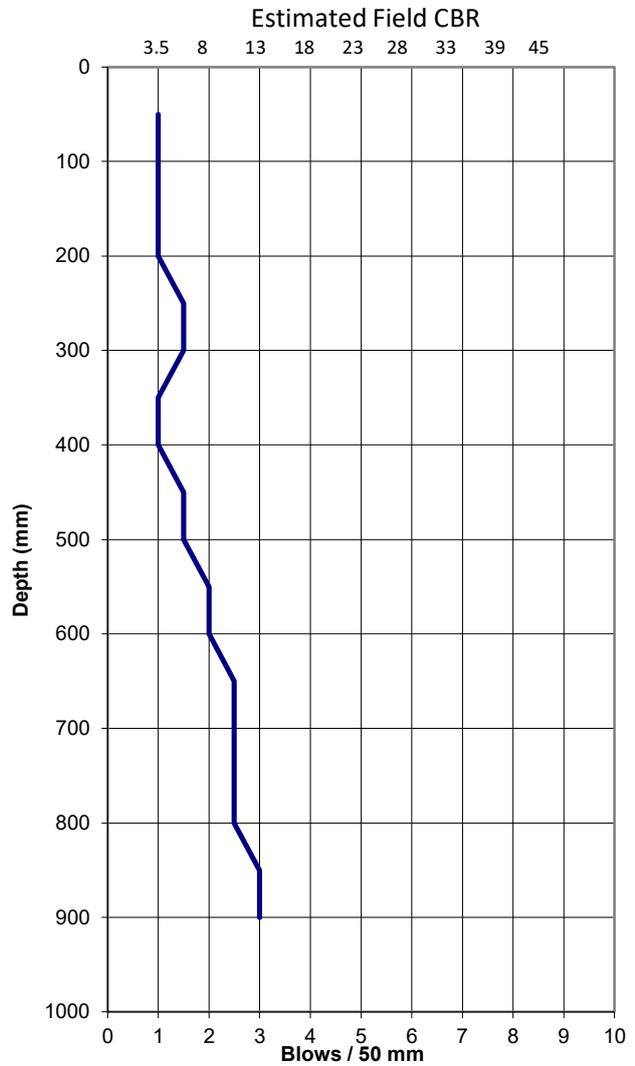


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP26 Sheet 27 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	1
100	1
150	1
200	1
250	1.5
300	1.5
350	1
400	1
450	1.5
500	1.5
550	2
600	2
650	2.5
700	2.5
750	2.5
800	2.5
850	3
900	3



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

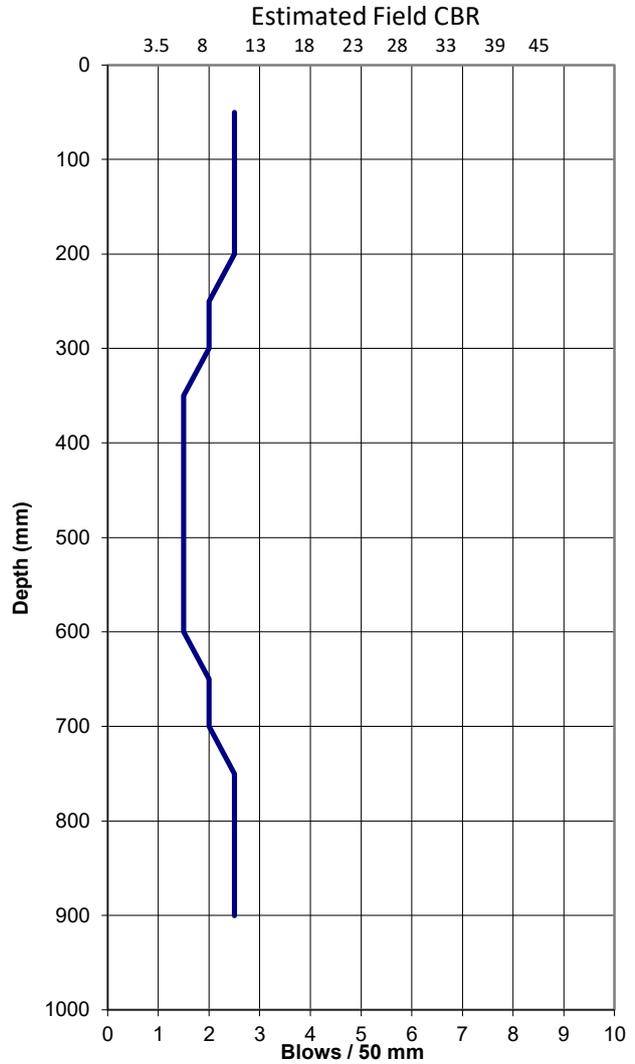


Piritahi

SCALA PENETROMETER LOG

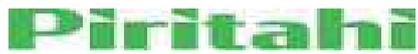
Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP27 Sheet 28 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	2.5
100	2.5
150	2.5
200	2.5
250	2
300	2
350	1.5
400	1.5
450	1.5
500	1.5
550	1.5
600	1.5
650	2
700	2
750	2.5
800	2.5
850	2.5
900	2.5



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

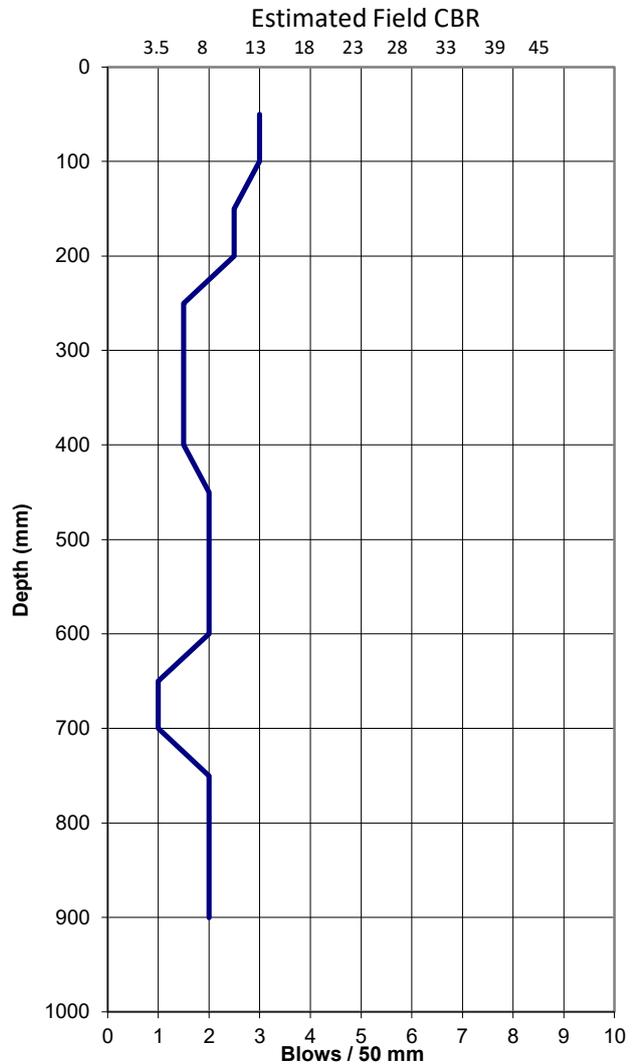


Piritahi

SCALA PENETROMETER LOG

Job No: 1007708.2086	Date: 23/02/2023	Test No. DCP28 Sheet 29 of 29
Project: Piritahi Aorere	Operated by: SCZH+CATA	
Location: AO-16	Logged by: SCZH+CATA	
RL: 17.5	Checked by:	

mm Driven	No. of Blows
50	3
100	3
150	2.5
200	2.5
250	1.5
300	1.5
350	1.5
400	1.5
450	2
500	2
550	2
600	2
650	1
700	1
750	2
800	2
850	2
900	2



Note: The estimated CBR values are based upon Fig. 5, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (1992) 'Pavement Design - A Guide to the Structural Design of Road Pavements'

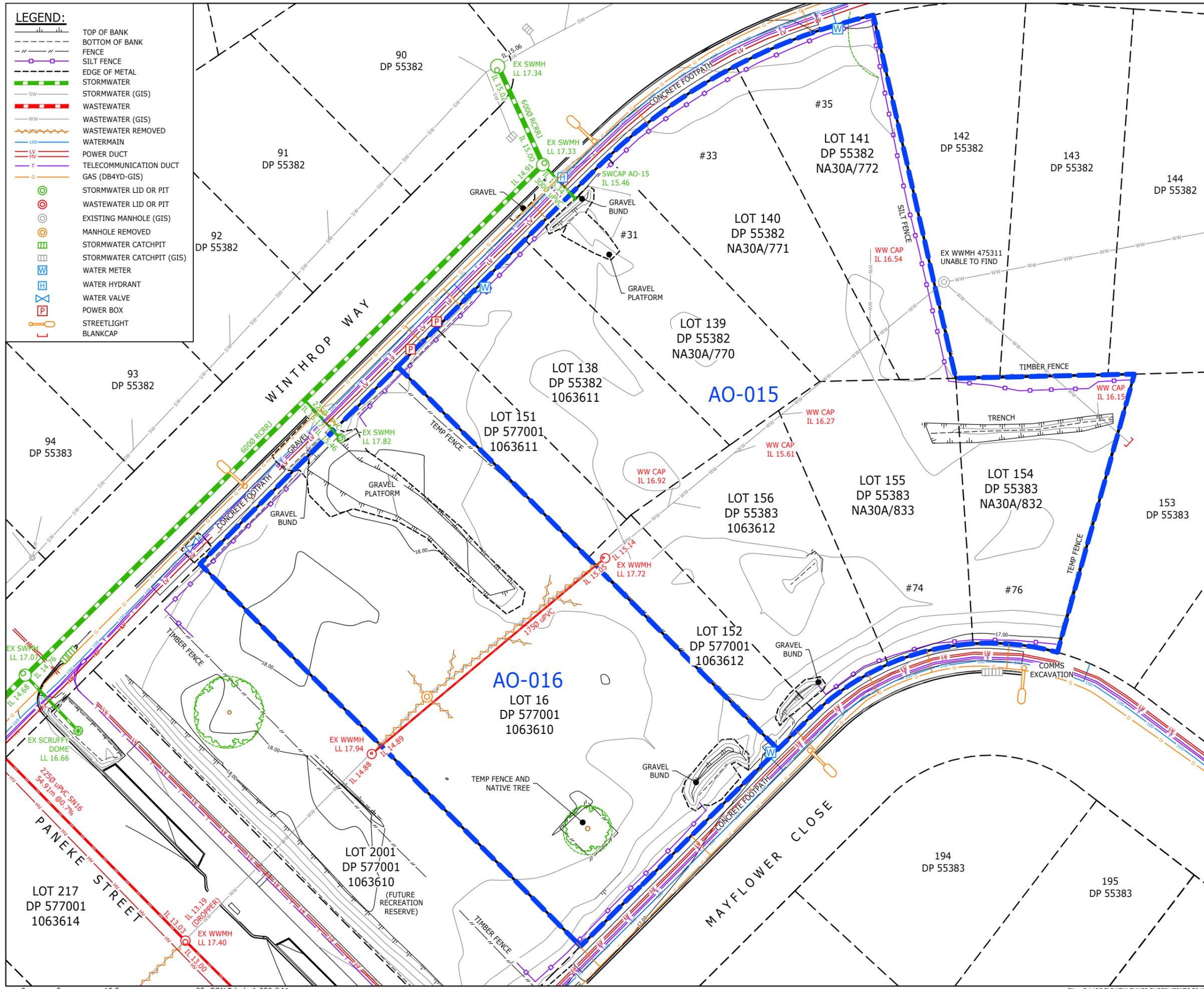
Test Method Used: NZS 4402:1988 Test 6.5.2 Dynamic Cone Penetrometer

Appendix B – As Built Plans

LEGEND:

	TOP OF BANK
	BOTTOM OF BANK
	FENCE
	SILT FENCE
	EDGE OF METAL STORMWATER
	STORMWATER (GIS)
	WASTEWATER
	WASTEWATER (GIS)
	WASTEWATER REMOVED
	WATERMAIN
	POWER DUCT
	TELECOMMUNICATION DUCT
	GAS (DB4YD-GIS)
	STORMWATER LID OR PIT
	WASTEWATER LID OR PIT
	EXISTING MANHOLE (GIS)
	MANHOLE REMOVED
	STORMWATER CATCHPIT
	STORMWATER CATCHPIT (GIS)
	WATER METER
	WATER HYDRANT
	WATER VALVE
	POWER BOX
	STREETLIGHT
	BLANKCAP

- NOTES:**
- LEVELS ARE IN TERMS OF AUCKLAND VERTICAL DATUM 1946
ORIGIN OF LEVELS
S166 SO 55792 (CA09)
RL 17.38
 - COORDINATES ARE IN TERMS OF NZ GEODETIC DATUM 2000
MT EDEN CIRCUIT
ORIGIN OF COORDINATES
S166 SO 55792 (CA09)
789350.09mN
405249.75mE
 - CONTOURS ARE AT 0.2m INTERVALS. CONTOURS SHOWN WITHIN THE SURVEYED AREA HAVE BEEN ELECTRONICALLY COMPUTED FROM SPOT HEIGHT DETERMINATIONS AND MAY NOT REPRESENT THE TRUE GROUND LEVELS. CONTOURS SHOWN OUTSIDE THE SURVEYED AREA ARE SOURCED FROM AUCKLAND COUNCIL LIDAR & VERIFIED BY PIRITAHĪ TO WITHIN +/- 50mm. ANY CRITICAL HEIGHTS SHOULD BE CHECKED ON SITE PRIOR TO DESIGN AND CONSTRUCTION COMMENCING.
 - THIS PLAN HAS BEEN CARRIED OUT TO TOPOGRAPHICAL STANDARDS. ALL LEVELS SHOWN ARE CORRECT AT TIME OF SURVEY. CRITICAL DIMENSIONS AND LEVELS SHOULD BE VERIFIED.
 - BOUNDARIES SHOWN ON THIS PLAN ARE FROM LAND INFORMATION NZ DCDB AND HAVE NOT BEEN SURVEYED. A BOUNDARY DEFINITION SURVEY SHOULD BE CARRIED OUT TO ESTABLISH EXACT BOUNDARY POSITIONS ON SITE.
 - ALL EASEMENTS, COVENANTS AND OTHER LEGAL INSTRUMENTS ASSOCIATED WITH THIS SITE ARE CORRECT AS AT THE DATE OF ISSUE OF THIS DRAWING.
 - REFER TO DRAWING FILE FOR FROZEN LAYERS : SPOT HEIGHTS AND 3D SURFACE.



1	FOR HANDOVER	RBH	22.03.2023
REVISION DETAILS		BY	DATE

PROJECT: **AORERE STAGE 2**

DESCRIPTION: **SUPERLOT AO-15 AND AO-16 HANDOVER PLAN**

SURVEYED	MJS	03.2023
DESIGNED	-	-
DRAWN	CA	03.2023
CHECKED	RBH	22.03.2023
APPROVED	RBH	22.03.2023

SCALE: 1:250 @A1 | 1:500 @A3 | REVISION: **1**

STATUS: FOR HANDOVER
PRECINCT: MĀNGERE
DWG NO: **TOC045-A01-1516**



Appendix C – Piritahi Test Results

CHECKLIST FOR REMOVING REDUNDANT SERVICE AND BACKFILLING

TOC No & Name:	TOC45, AORERE	Physical Work Start Date:	6/04/2022
Location/Description:	A0-016, Existing Services Removal		

Activities/Items to be checked: *Discipline: Removing old Services*

EPA NO.: N/A

NO	Activity/Item	Checked OK? (Y/N/NA)	Checked By	Remarks/ Hold Point Initials
0	Drawings No.: - Drawing Rev.: -	Yes	Harjot	-
1	Dig permit in place, readily available and current	Yes	Harjot/ David	Dig permit was reviewed and discussed with the operator and spotter
2	Underground services identified and exposed on site and photos taken.	Yes	Harjot	See photos attached below
3	Unsuitable/suitable material identified by CPS prior to excavation	Yes	Harjot	Photos of capped lot connections attached
4	Identified material and the redundant services/drainage features/lot connections/pipe(s) removed and photos taken.	Yes	Harjot	WW lines were As-built and capped 1 meter of the main line
5	Trench extents of redundant services /drainage features /lot connection/pipe(s) surveyed and presented on as-builts, with the exclusion of lot connections that are less than 600mm deep.	Yes	Blago Mitevski	
6	Exposed trench foundation inspected by CPS/Designer and Shear Vane and Scala tested as per Item 2.3 of AAAA-DS-ITP-0002 Earthworks - Fill prior to backfilling - CPS Hold Point		CPS	CPS Initial: _____
7	Backfill material test results meet the requirements of Item 1.1 of AAAA-DS-ITP-0002 Earthworks - Fill (IANZ test report required) - Backfilling to be recorded by suitable photos.	Yes	Harjot	Backfill Material Type: Cohesive Backfill

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CHECKLIST FOR REMOVING REDUNDANT SERVICE AND BACKFILLING

8	Trench backfilling compaction tested in layers and acceptable in accordance with Item 2.7 of ITP-DS-ITP-0002 Earthworks - Fill (Shear Vane/Clegg/ NDM test report required)	Yes	Harjot & Geotechnics	Shear Vanes and NDMs for laterals at a depth of greater than 600 mm
9	Stabilisation of final surfaces finished and inspected by CPS - CPS Hold Point		CPS	CPS Initial: _____

Verification and Review:

I confirm that the above work has been completed and checked as required.

Project/Site Engineer Name: Harjot Singh Signature: Harjot Singh Date: 25/10/2022

I confirm that QA checks for the above work have been completed and compliant.

QA Engineer Name: Signature: Date:

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CHECKLIST FOR REMOVING REDUNDANT SERVICE AND BACKFILLING

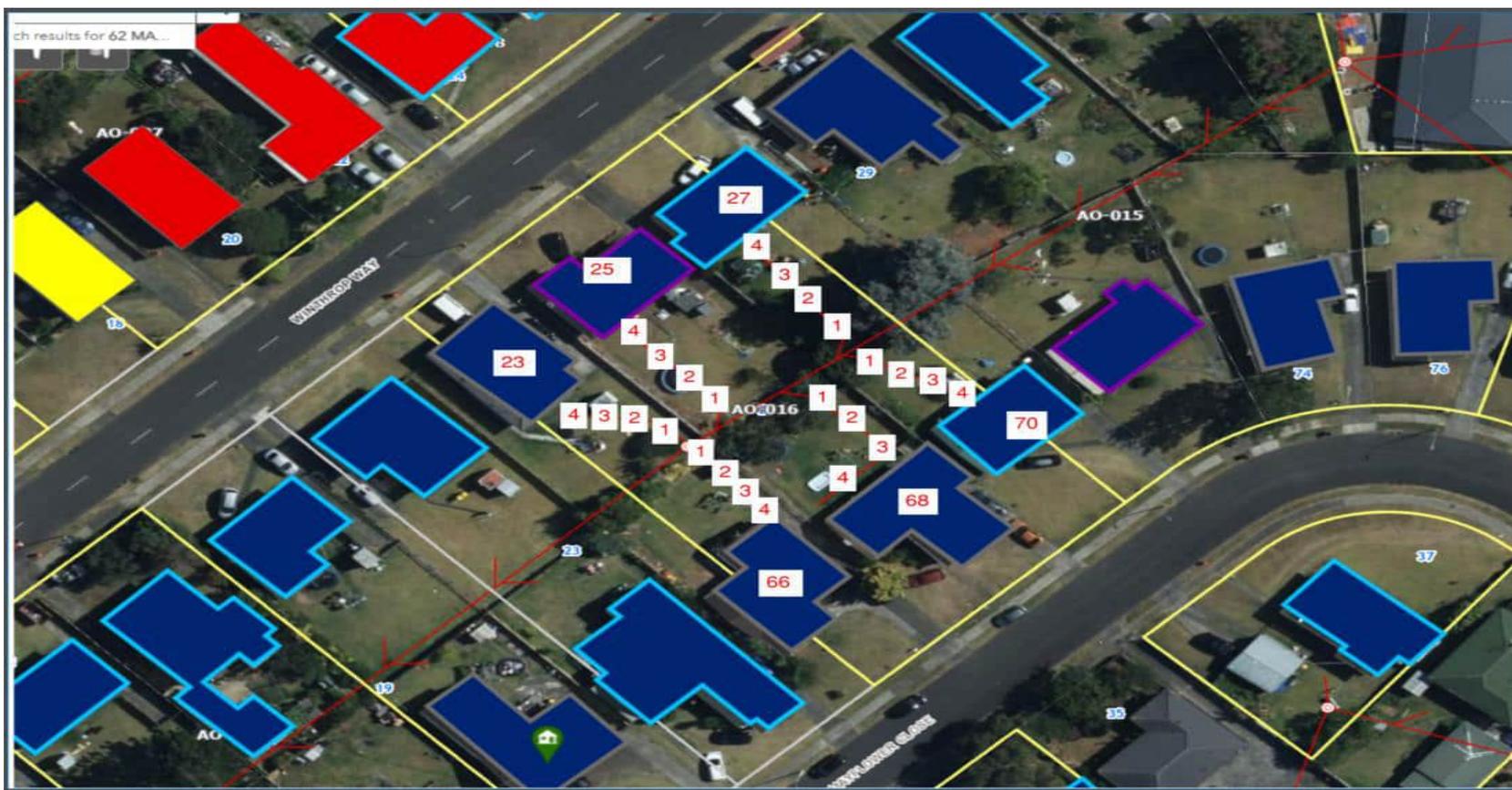
Test Location Number	Location (Or Show on Site Plan)		Shear Vane Uncorrected Dial Readings (kPa) Waste-Water removed laterals					Corrected Average Shear Strength (kPa)
	Dist. from DS MH (m)	Test R. L. /Depth(m)	Reading 1	Reading 2	Reading 3	Reading 4	Average Reading	
66 Mayflower	n/a	n/a	140	140	102	140	130.5	173.6
68 Mayflower	n/a	n/a	140	140	140	105	131.25	174.6
70 Mayflower	n/a	n/a	140	140	140	140	140	186.2
23 Winthrop Way	n/a	n/a	115	140	140	130	131.25	174.6
25 Winthrop Way	n/a	n/a	140	140	100	140	130	172.9
27 Winthrop Way	n/a	n/a	112	126	140	140	129.5	172.2
			Tested By: Harjot. S		Date: 12/04/2022			
			Reviewed By: Amrit. S		Date: 25/10/2022			

Please Note – The SW laterals we removed for these properties were inside the topsoil or in the first 200 mm depth of the clay therefore no testing was undertaken for the SW redundant lines that were removed.

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CHECKLIST FOR REMOVING REDUNDANT SERVICE AND BACKFILLING

Site Plan for shear vane testing



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CHECKLIST FOR REMOVING REDUNDANT SERVICE AND BACKFILLING



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CHECKLIST FOR REMOVING REDUNDANT SERVICE AND BACKFILLING



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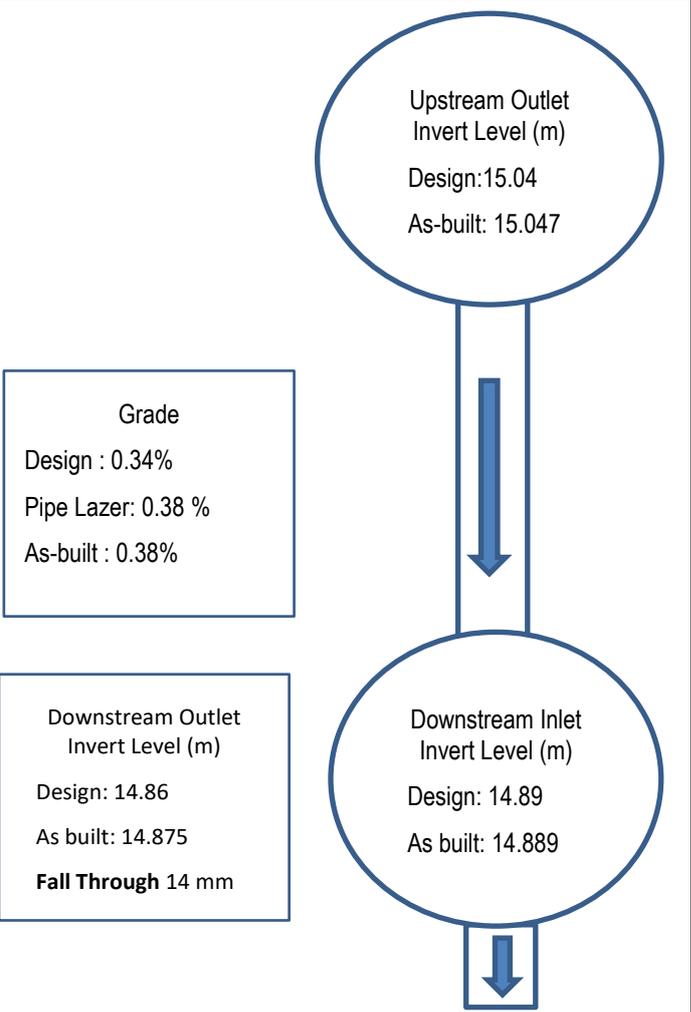
CHECKLIST FOR REMOVING REDUNDANT SERVICE AND BACKFILLING

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CHECKLIST FOR DRAINAGE PIPELINE AND MANHOLE INSTALLATION

TOC No & Name:	Toc 45, Aorere	Physical Work Start Date:	13-06-2022
Location/Description:	A0-016, AO-015 WWMH C1 TO WWMH C-2		
IFC Drawing No:	TOC045-002-5002	Revision:	A

Upstream Manhole (MH) #: MH C-2	
Status	New Existing
Dia. (mm)	1200
Type	SW WW
Epoxied	YES
Water tight	YES
Depth	2.82
Note: Depth is measured from MH Base to underside of Concrete Lid.	
Throat depth	N/A
Note: Throat depth is measured from underside of concrete lid to finished final lid level (FSL)	
Pipe Line Ref #: LINE C	
Dia. (mm)	150
Length (m)	42.02
Material	UPVC
Class	SN16
Downstream Manhole (MH) #: MH C-1	
Status	New Existing
Dia. (mm)	1200
Type	SW WW
Epoxied	YES
Water tight	YES
Depth	3.21
Note: as above	
Throat depth	N/A
Note: as above	



Activities/Items to be checked: **Discipline:** **EPA NO.:**

NO	Activity/Item	Checked OK? (Y/N/NA)	Checked By	Remarks/ Hold Point Initials
1	Underground services identified and marked out	Yes	Amrit, Harjot	Marked out on site + on the B4UDIG plans
2	Setting out of the manhole (position and level) completed and fully understood	Yes	Mick Lindsay	Surveyor

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CHECKLIST FOR DRAINAGE PIPELINE AND MANHOLE INSTALLATION

NO	Activity/Item	Checked OK? (Y/N/NA)	Checked By	Remarks/ Hold Point Initials
3	Trench formation/foundation inspected and any unsuitable material identified by CPS/Designer - CPS Hold Point	Yes	CPS	CPS Initial:
4	Trench formation/foundation shear vane tested	Yes	Harjot/ Amrit	See site test records attached
5	Undercut inspected (if applicable): <ul style="list-style-type: none"> Undercut Depth: 0.5 m Undercut Type: GAP65 Material Source : Stevensons Aggregate 	Yes	Geotech- NIBU	Corrected shear vane results ranged from 20 kpa- 40 kpa within the alluvium therefore a 500mm as per WW ITP was completed
6	Bedding inspected and clegg tested <ul style="list-style-type: none"> Bedding Depth: 100 mm Bedding Type: Gap 20 Material Source: Stevensons Aggregate 	Yes	Amrit / Harjot	See site test records attached
7	Pipes and manholes inspected and free of any damage	Yes	Amrit/ Harjot	Yes, free of any damage
8	Pipes and manholes installed as per design drawings and specification - CPS Hold Point	Yes	CPS	CPS Initial:
9	Backfill compacted in layers and tested	Yes	Amrit/Harjot	IANZ NDMs completed & Shear vane see below.
10	Test results reviewed and compactness achieved for (Photo required): <input checked="" type="checkbox"/> Foundation <input checked="" type="checkbox"/> Bedding <input checked="" type="checkbox"/> Backfilling	Yes	Amrit/Harjot	Photos attached

Photos below taken and attached (Tick Applicable)

<input checked="" type="checkbox"/> Flexible Joints	<input checked="" type="checkbox"/> Pipe Joint	<input checked="" type="checkbox"/> Excavation
<input checked="" type="checkbox"/> Haunching	<input checked="" type="checkbox"/> Manhole benching	<input checked="" type="checkbox"/> Undercut if required

Verification and Review:

I confirm that the above work has been completed and checked as required.

Subcontractor	Name: Dean Strong	Signature:	Date:
PE/SE (Piritahi)	Name: Harjot Singh	Signature: H.S	Date: 19/08/2022
Quality Engineer	Name:	Signature:	Date:

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SHEAR VANE FIELD TEST RECORD SHEET

TOC No & Name:	Aorere, TOC 45	Date:	30/01/2023
Location:	WWMH C1 – C2	Layer:	250mm

Test Methods:

Shear Vane

Criteria – Shear Vane ≥ 130 kPa and NDM with Air Voids $< 8\%$

Shear Vane every 250 layer, NDM every 500mm layer (approximately)

Material Type: Cohesive Fill	Shear Vane ID: 3048
Material Description: Clay	Calibration Expiry Date: 14/10/2023
Site Plan Attached: See Geotech NDM Report	Vane Conversion Factor: 1.33

Test Location Number	Location (Or Show on Site Plan)		Shear Vane Uncorrected Dial Readings (kPa)					Corrected Average Shear Strength (kPa)
	Dist. from DS MH (m)	Test R. L. /Depth(m)	Waste-Water Lateral 1				Average Reading	
			Reading 1	Reading 2	Reading 3	Reading 4		
1	10	0.2	120	122	110	130	120.5	160.3
2	20	0.2	108	112	120	110	112.5	149.6
3	30	0.2	140	140	138	140	139.5	185.5
4	40	0.2	104	100	110	106	105	139.7
1	10	0.4	116	120	110	108	113.5	150.9
2	20	0.4	110	118	120	120	117	155.6
3	30	0.4	108	110	110	108	109	144.9
4	40	0.4	108	100	112	110	107.5	142.9
1	10	0.6	110	110	118	120	114.5	152.28
2	20	0.6	126	130	128	130	128.5	170.9
3	30	0.6	120	114	110	112	114	151.62
4	40	0.6	108	108	120	126	115.5	153.6
1	10	0.8	130	128	108	108	118.5	157.6
2	20	0.8	122	110	110	110	113	150.29
3	30	0.8	118	112	120	126	119	158.3
4	40	0.8	110	110	110	108	109.5	145.6
1	10	1.0	112	120	120	110	115.5	153.6

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SHEAR VANE FIELD TEST RECORD SHEET

2	20	1.0	110	108	130	128	119	158.3
3	30	1.0	120	120	122	110	118	156.9
4	40	1.0	110	110	108	108	109	144.9
1	10	1.2	126	130	110	110	119	158.2
2	20	1.2	120	122	120	120	120.5	160.2
3	30	1.2	110	108	122	110	112.5	149.6
4	40	1.2	120	122	108	108	114.5	152.2
1	10	1.4	110	108	126	130	118.5	157.6
2	20	1.4	120	120	120	122	120.5	160.3
3	30	1.4	108	108	112	110	109.5	145.6
4	40	1.4	126	130	136	132	131	174.23
1	10	1.6	120	120	122	120	120.5	160.2
2	20	1.6	112	110	108	110	110	146.3
3	30	1.6	130	136	120	122	127	168.91
4	40	1.6	120	122	110	108	115	152.9
1	10	1.8	110	108	112	120	112.5	149.6
2	20	1.8	136	120	122	120	124.5	165.6
3	30	1.8	110	108	126	120	116	154.3
4	40	1.8	136	120	110	108	118.5	157.6
1	10	2.0	110	110	116	120	114	151.6
2	20	2.0	136	108	136	108	122	162.26
3	30	2.0	110	120	110	120	115	152.9
4	40	2.0	120	130	132	130	128	170.24

Link for NDM (Hardfill Backfill)-

5 MANGERE/TOC045 - Aorere Stage 2/3C/04 QAQC Records/04.02 Construction Checklists/04.02e Wastewater/NDM Hardfill WWMH C1- C2.pdf

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SHEAR VANE FIELD TEST RECORD SHEET

Link for NDM (Cohesive Backfill) -

5 MANGERE/TOC045 - Aorere Stage 2/3C/04 QAQC Records/04.02 Construction Checklists/04.02e Wastewater/NDM WWMH C1 - C2 L1 & L2.pdf

5 MANGERE/TOC045 - Aorere Stage 2/3C/04 QAQC Records/04.02 Construction Checklists/04.02e Wastewater/WWMH C1- C2 NDM L3.pdf

5 MANGERE/TOC045 - Aorere Stage 2/3C/04 QAQC Records/04.02 Construction Checklists/04.02e Wastewater/NDM WWMH C1-C2 L4 & L5.pdf

Tested By: Harjot. S

Date: 16.02.2023

Reviewed By:

Date:

SHEAR VANE FIELD TEST RECORD SHEET



Jul 4, 2022
3:18 PM UTC +12:00
C1 to C2
Draeinail Construction
Piritahi

SHEAR VANE FIELD TEST RECORD SHEET



Jul 4, 2022
10:27 AM UTC +12:00
C1 to C2
Draeinail Construction
Piritahi

SHEAR VANE FIELD TEST RECORD SHEET



Jul 4, 2022
12:48 PM UTC +12:00
Undercut compaction of
Drainail Construction
Piritahi

SHEAR VANE FIELD TEST RECORD SHEET



Jun 28, 2022
4:07 PM UTC +12:00
Concrete haunching of C...
Draeinail Construction
Piritahi

SHEAR VANE FIELD TEST RECORD SHEET



SHEAR VANE FIELD TEST RECORD SHEET



SHEAR VANE FIELD TEST RECORD SHEET



Jul 5, 2022
9:40 AM UTC +12:00
C2 outlet
Draeinail Construction
Piritahi

SHEAR VANE FIELD TEST RECORD SHEET



Jul 5, 2022
10:13 AM UTC +12:00
C1 outlet haunching
Draeinail Construction
Piritahi

SHEAR VANE FIELD TEST RECORD SHEET



Jul 1, 2022

9:20 AM UTC +12:00

Geo cloth and grid C1 t...

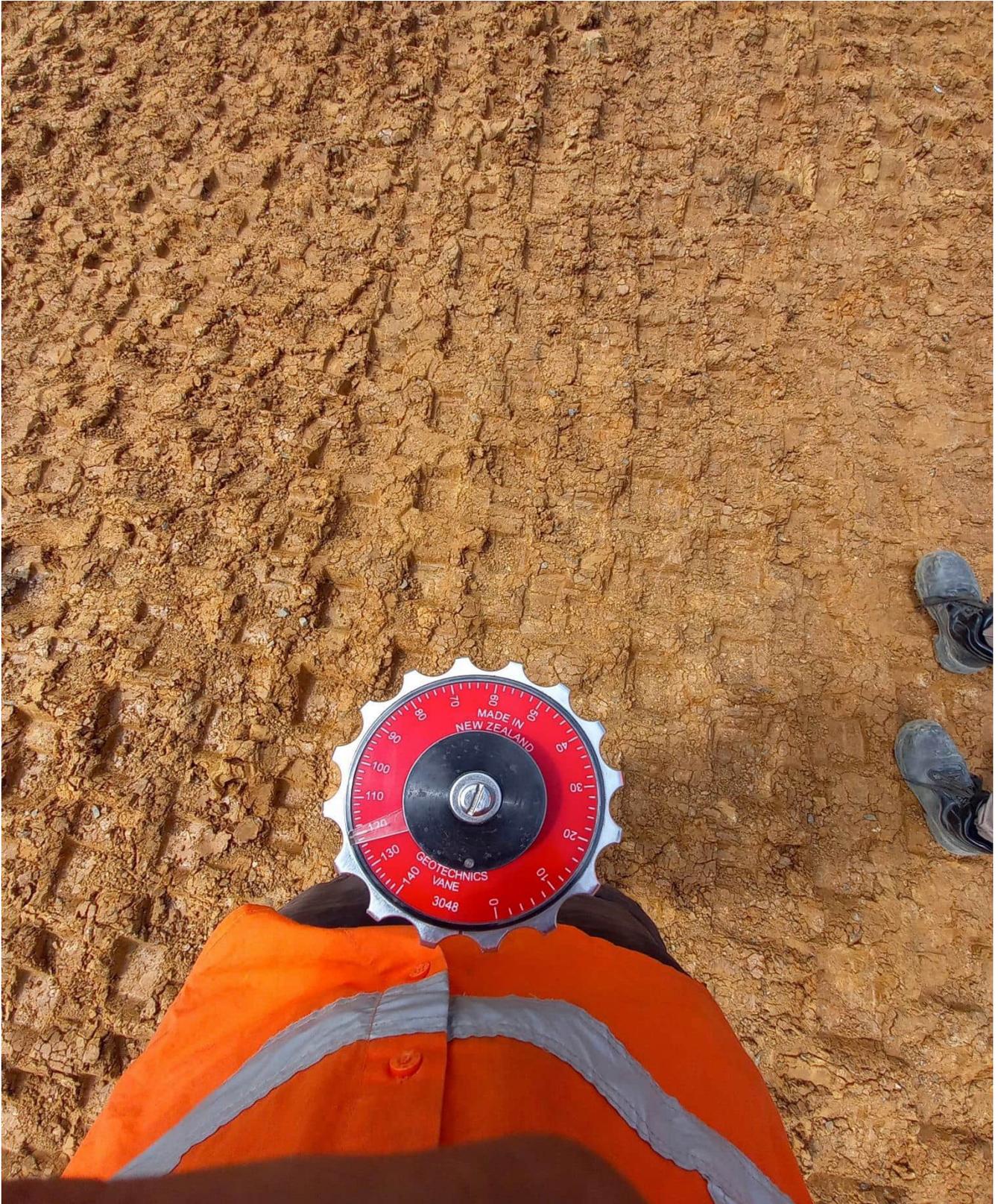
Drainail Construction

Piritahi

SHEAR VANE FIELD TEST RECORD SHEET

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SHEAR VANE FIELD TEST RECORD SHEET



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CLEGG HAMMER TEST RECORD SHEET



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CLEGG HAMMER TEST RECORD SHEET

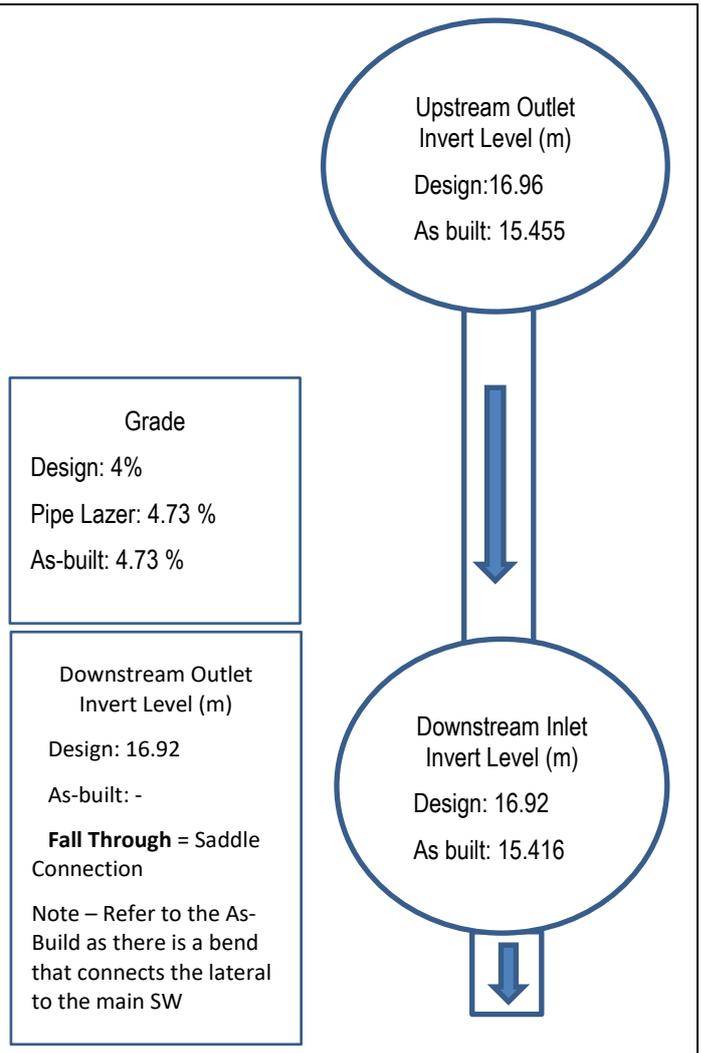
CHECKLIST FOR DRAINAGE PIPELINE AND MANHOLE INSTALLATION

TOC No & Name:	Toc 45, Aorere	Physical Work Start Date:	6-10-2022
Location/Description:	A0-016 – Winthrop Way MH A0-16 – Saddle 2-12		
IFC Drawing No:	TOC045-002-4003	Revision:	A

Upstream Manhole (MH) #: MH A0-016	
Status	New Existing
Dia. (mm)	1050
Type	SW WW
Epoxied	YES
Water tight	YES
Depth	2.44
Note: Depth is measured from MH Base to underside of Concrete Lid.	
Throat depth	N/A
Note: Throat depth is measured from underside of concrete lid to finished final lid level (FSL)	

Pipe Line Ref #: LINE A0-16	
Dia. (mm)	225
Length (m)	6.10m
Material	UPVC
Class	SN16

Downstream Saddle #: 2-12	
Status	New Existing
Dia. (mm)	225
Type	SW WW
Epoxied	YES
Water tight	YES
Depth	2.53
Note: as above	
Throat depth	N/A
Note: as above	



Activities/Items to be checked: *Discipline: Storm Water* *EPA NO.: EPA - 132180*

NO	Activity/Item	Checked, OK? (Y/N/NA)	Checked By	Remarks/ Hold Point Initials
1	Underground services identified and marked out	Yes	Amrit, Harjot	Marked out on site + on the B4UDIG plans
2	Setting out of the manhole (position and level) completed and fully understood	Yes	John Bucholz	Surveyor

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CHECKLIST FOR DRAINAGE PIPELINE AND MANHOLE INSTALLATION

NO	Activity/Item	Checked, OK? (Y/N/NA)	Checked By	Remarks/ Hold Point Initials
3	Trench formation/foundation inspected and any unsuitable material identified by CPS/Designer - CPS Hold Point	Yes	CPS	CPS Initial:
4	Trench formation/foundation shear vane tested	Yes	Harjot/ Amrit	See site test records attached
5	Undercut inspected (if applicable): <ul style="list-style-type: none"> Undercut Depth: 0.5 m Undercut Type: GAP65 Material Source : Stevensons 	No		
6	Bedding inspected and clegg tested): <ul style="list-style-type: none"> Bedding Depth: 100 mm Bedding Type: Gap 20 Material Source: Stevensons Aggregate 	Yes	Amrit / Harjot	See site test records attached
7	Pipes and manholes inspected and free of any damage	Yes	Amrit/ Harjot	Yes, free of any damage
8	Pipes and manholes installed as per design drawings and specification - CPS Hold Point	Yes	CPS	CPS Initial:
9	Backfill compacted in layers and tested	Yes	Amrit/Harjot	See site test records attached and/or IANZ Test Reports
10	Test results reviewed and compactness achieved for (Photo required): <input checked="" type="checkbox"/> Foundation <input checked="" type="checkbox"/> Bedding <input checked="" type="checkbox"/> Backfilling	Yes	Amrit/Harjot	Photos attached

Photos below taken and attached (Tick Applicable)

<input checked="" type="checkbox"/> Flexible Joints	<input checked="" type="checkbox"/> Pipe Joint	<input checked="" type="checkbox"/> Excavation
<input checked="" type="checkbox"/> Haunching	<input checked="" type="checkbox"/> Manhole benching	<input checked="" type="checkbox"/> Undercut if required

Verification and Review:

I confirm that the above work has been completed and checked as required.

Subcontractor **Name: Shea. F** **Signature:** **Date: 10/11/2022**

PE/SE (Piritahi) **Name: Harjot Singh** **Signature: H.S** **Date: 10/11/2022**

Quality Engineer **Name:** **Signature:** **Date:**

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CLEGG HAMMER TEST RECORD SHEET

TOC No & Name:	TOC 45, AORERE	Date:	
Location:	SW Saddle 2-12 to MH A0-016	Layer:	0.2

Test Methods:

Material Type: CLAY	Shear Vane ID: S/NO 3048
Material Description: Clay	Calibration Expiry Date: 14/10/2023
Site Plan Attached: N/a	Vane Conversion Factor: 1.33

Test Methods:

Material Type:	Gap 20	Clegg Hammer ID:	S/NO 20-0055
Material Description:	Hardfill	Calibration Expiry Date:	31/10/2023
Criteria:	Gap20 CIV > 12	Drops:	4

Test Methods:

Material Type:	Gap 65	Clegg Hammer ID:	S/NO 20-0055
Material Description:	Hardfill	Calibration Expiry Date:	31/10/2023
Criteria:	Gap65 CIV > 30 and no single value < 28, in 0.2 m Layers	Drops:	4

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CLEGG HAMMER TEST RECORD SHEET

Testing Equipment Information		
<i>Glegg Hammer ID</i>	<i>Shear Vane ID</i>	<i>Shear Vane Correction Factor</i>
S/NO 20-0055	S/NO 3048	1.33

Trench Foundation Test Records (Correction Factor not applied)								
	<i>Material</i>	<i>Saddle</i>	<i>20m</i>	<i>40m</i>	<i>45m</i>	<i>60m</i>	<i>75m</i>	<i>U/s MH</i>
<i>Shear Vane</i>	CLAY	2-12	102	N/A	N/A	N/A	N/A	MH A0-16

Material Type and Source					
<i>Undercut Depth</i>	-	<i>Type</i>	<i>Gap65</i>	<i>Source</i>	STEVENSON'S
<i>Bedding Depth</i>	0.1m	<i>Type</i>	<i>Gap20</i>	<i>Source</i>	STEVENSON'S
<i>Backfill Depth</i>	2.53 m	<i>Type</i>	<i>Gap65</i>	<i>Source</i>	STEVENSON'S
<i>Undercut Link – C.P.S</i>					

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CLEGG HAMMER TEST RECORD SHEET

Backfill Test Record (CLEGG) MH A0-016- Saddle 2-12

<u>Test Type</u>	<u>Depth</u>	<u>D/s Saddle</u>	<u>0-15m</u>					<u>15-30m</u>					<u>30-45m</u>					<u>U/s MH</u>
			<u>T1</u>	<u>T2</u>	<u>T3</u>	<u>T4</u>		<u>T1</u>	<u>T2</u>	<u>T3</u>	<u>T4</u>		<u>T1</u>	<u>T2</u>	<u>T3</u>	<u>T4</u>		
		<u>Saddle</u>																<u>MH A0-016</u>
Granular (Clegg)	Bedding	2-12	14	13	13	12	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	<u>MH A0-016</u>
Granular (Clegg)	0.0 (1m ap)	2-12	32	33	31	34	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	<u>MH A0-016</u>
Granular (Clegg)	0.2m	2-12	34	33	34	35	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	<u>MH A0-016</u>
Granular (Clegg)	0.4m	2-12	31	31	33	32	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	<u>MH A0-016</u>
Granular (Clegg)	0.6m	2-12	33	34	33	33	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	<u>MH A0-016</u>
Granular (Clegg)	0.8m	2-12	34	33	34	33	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	<u>MH A0-016</u>
Granular (Clegg)	1.0m	2-12	32	34	33	33	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	<u>MH A0-016</u>
Granular (Clegg)	1.2m	2-12	32	34	33	33	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	<u>MH A0-016</u>
Granular (Clegg)	1.4m	2-12	30	32	33	31	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	<u>MH A0-016</u>
Granular (Clegg)	1.6m	2-12	34	33	34	31	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	<u>MH A0-016</u>
Granular (Clegg)	3.2m	2-12	33	34	33	34	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	MH A0-017

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CLEGG HAMMER TEST RECORD SHEET

Backfill Test Record (CLEGG) MH A0-016

<u>Test Type</u>	<u>Depth</u>	<u>D/s Saddle</u>	<u>0-15m</u>					<u>15-30m</u>					<u>30-45m</u>					
			<u>T1</u>	<u>T2</u>	<u>T3</u>	<u>T4</u>		<u>T1</u>	<u>T2</u>	<u>T3</u>	<u>T4</u>		<u>T1</u>	<u>T2</u>	<u>T3</u>	<u>T4</u>		
Granular (Clegg)	Bedding	<u>MH A0-016</u>	13	13	13	14	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Granular (Clegg)	0.0 (1m ap)	<u>MH A0-016</u>	34	33	31	33	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Granular (Clegg)	0.2m	<u>MH A0-016</u>	32	33	34	33	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Granular (Clegg)	0.4m	<u>MH A0-016</u>	34	33	33	33	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Granular (Clegg)	0.6m	<u>MH A0-016</u>	33	34	33	34	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Granular (Clegg)	0.8m	<u>MH A0-016</u>	33	32	33	32	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Granular (Clegg)	1.0m	<u>MH A0-016</u>	32	34	32	34	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Granular (Clegg)	1.2m	<u>MH A0-016</u>	33	32	34	33	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Granular (Clegg)	1.4m	<u>MH A0-016</u>	32	34	32	32	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Granular (Clegg)	1.6m	<u>MH A0-016</u>	34	32	32	34	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Granular (Clegg)	1.8m	<u>MH A0-016</u>	33	34	33	32	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

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CLEGG HAMMER TEST RECORD SHEET

SHEAR VANE FIELD TEST RECORD SHEET

TOC No & Name:	Aorere, TOC 45	Date:	13-06-2022
Location:	WWMH C1	Layer:	250mm

Test Methods: Shear Vane

Criteria – Shear Vane \geq 130 kPa Shear Vane every 250 layer

Material Type: Cohesive Fill	Shear Vane ID: 3048
Material Description: Clay	Calibration Expiry Date: 14/10/2023
Site Plan Attached: Yes	Vane Conversion Factor: 1.33

WWMH
C1

Test Location Number	Location (Or Show on Site Plan)		Shear Vane Uncorrected Dial Readings (kPa) Waste-Water Lateral 1					Corrected Average Shear Strength (kPa)
	Dist. from DS MH (m)	Test R. L. /Depth(m)	Reading 1	Reading 2	Reading 3	Reading 4	Average Reading	
1	0	0.2	100	110	106	105	105	139.5
	0	0.2	120	110	110	118	114.5	152.3
2	0	0.2	130	128	110	108	119	154.7
	0	0.2	114	110	112	110	119	154.7
3	0	0.4	108	120	118	120	116.5	155
	0	0.4	126	130	128	130	128.5	171
4	0	0.6	120	110	118	112	115	152
	0	0.6	108	130	128	126	123	163.6
5	0	0.8	104	108	108	106	106.5	141.6
	0	0.8	116	130	128	108	106.5	141.6
6	0	1.0	110	122	110	120	115.5	153.6
	0	1.0	108	118	112	108	111.5	148.3
7	0	1.2	108	108	108	110	108.5	144.3
	0	1.2	120	116	120	130	121.5	161.6
8	0	1.4	108	110	118	110	111.5	148.3
	0	1.4	140	108	110	140	124.5	165.6

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SHEAR VANE FIELD TEST RECORD SHEET

9	0	1.6	104	116	120	106	111.5	148.3
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Test Location Number	Location (Or Show on Site Plan)		Shear Vane Uncorrected Dial Readings (kPa) Waste-Water Lateral 2					Corrected Average Shear Strength (kPa)
	Dist. from DS MH (m)	Test R. L. /Depth(m)	Reading 1	Reading 2	Reading 3	Reading 4	Average Reading	
	0	1.6	136	122	110	108	119	158.3
10	0	1.8	110	108	122	110	112.5	149.6
	0	1.8	136	120	125	108	122.2	162.5
11	0	2.0	110	108	110	110	109.5	145.6
	0	2.0	110	108	120	120	114.5	152.3

Tested By: Harjot.S

Date: 16.02.2023

Reviewed By: Amrit .S

Date: 16.02.2023

WWMH C2

Test Location Number	Location (Or Show on Site Plan)		Shear Vane Uncorrected Dial Readings (kPa) Waste-Water Lateral 1					Corrected Average Shear Strength (kPa)
	Dist. from DS MH (m)	Test R. L. /Depth(m)	Reading 1	Reading 2	Reading 3	Reading 4	Average Reading	
1	0	0.2	110	108	122	110	112.5	149.6
	0	0.2	136	120	130	125	127.8	169.9
2	0	0.2	110	112	110	110	110.5	146.9
	0	0.2	110	108	120	120	114.5	152.3
3	0	0.4	110	122	110	120	115.5	153.6
	0	0.4	116	118	112	112	114.5	152.3

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SHEAR VANE FIELD TEST RECORD SHEET

4	0	0.6	112	114	110	110	111.5	148.3
	0	0.6	120	116	120	130	121.5	161.6
5	0	0.8	110	120	120	110	115	152.9
	0	0.8	120	110	112	112	113.5	150.9
6	0	1.0	110	128	120	120	119.5	158.9
	0	1.0	112	116	120	108	114	151.62
7	0	1.2	110	110	112	128	115	152.9
	0	1.2	120	120	114	124	119.5	158.9
8	0	1.4	120	116	120	130	121.5	161.5
	0	1.4	104	104	102	106	104	138.32
9	0	1.6	116	130	128	120	123.5	164.2

Test Location Number	Location (Or Show on Site Plan)		Shear Vane Uncorrected Dial Readings (kPa) Waste-Water Lateral 2					Corrected Average Shear Strength (kPa)
	Dist. from DS MH (m)	Test R. L. /Depth(m)	Reading 1	Reading 2	Reading 3	Reading 4	Average Reading	
	0	1.6	116	110	108	110	111	147.63
10	0	1.8	116	130	128	108	120.5	160.3
	0	1.8	110	122	110	120	115.5	153.6
11	0	2.0	108	118	112	108	111.5	148.3
	0	2.0	120	130	128	124	125.5	166.9

Tested By: Harjot.S

Date: 16.02.2023

Reviewed By: Amrit .S

Date: 16.02.2023

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CLEGG HAMMER TEST RECORD SHEET

TOC No & Name:	Toc 45, Aorere	Date:	13-06-2022
Location/Description	WWMH C1 – WWC2	Layer:	500mm & 250mm

Test Methods: Clegg Hammer

Material Type:	Granular Fill	Clegg Hammer ID:	20-0027
Material Description:	Gap 65	Calibration Expiry Date:	31-10-23
Criteria:	CIV >= 30	Drops:	4

Test Location Number	Location		Clegg Impact Value (CIV)	Test Location Number	Location		Clegg Impact Value (CIV)
	Dist. from DS MH (m)	Depth (m)			Dist. from DS MH (m)	Depth (mm)	
1	10	0.5	35	3	30	0.5	40
	10	0.5	32		30	0.5	42
	10	0.5	33		30	0.5	36
	10	0.5	33		30	0.5	38
2	20	0.5	32	4	40	0.5	32
	20	0.5	34		40	0.5	36
	20	0.5	35		40	0.5	32
	20	0.5	30		40	0.5	30
1	10	0.75	42	3	30	0.75	32
	10	0.75	36		30	0.75	36
	10	0.75	32		30	0.75	35
	10	0.75	36		30	0.75	30
2	20	0.75	35	4	40	0.75	32
	20	0.75	30		40	0.75	30
	20	0.75	42		40	0.75	32
	20	0.75	36		40	0.75	36
1	10	1.0	38	3	30	1.0	30
	10	1.0	32		30	1.0	32

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CLEGG HAMMER TEST RECORD SHEET

	10	1.0	32		30	1.0	38
	10	1.0	33		30	1.0	32
2	20	1.0	33	4	40	1.0	33
	20	1.0	38		40	1.0	33
	20	1.0	32		40	1.0	32
	20	1.0	36		40	1.0	34

Tested By: Harjot. S

Date: 27-06-2022

Reviewed By: Amrit. S

Date: 20-06-2022

RHS KERB FACING M/F CLOSE FROM ROAD 2

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Piritahi

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Document No.: AAAA-CN-FRM-0006

Revision: 1

Page 5 of 8

August 2021

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Jul 4, 2022
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Appendix D – Geotechnics Testing



1 Hill Street
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Project Name: Mangere Precinct TOC45
 Customer: Piritahi

Project Number	1041000.0045.1.0/
Entered By	KELF
Checked By	
Approved by KTP	

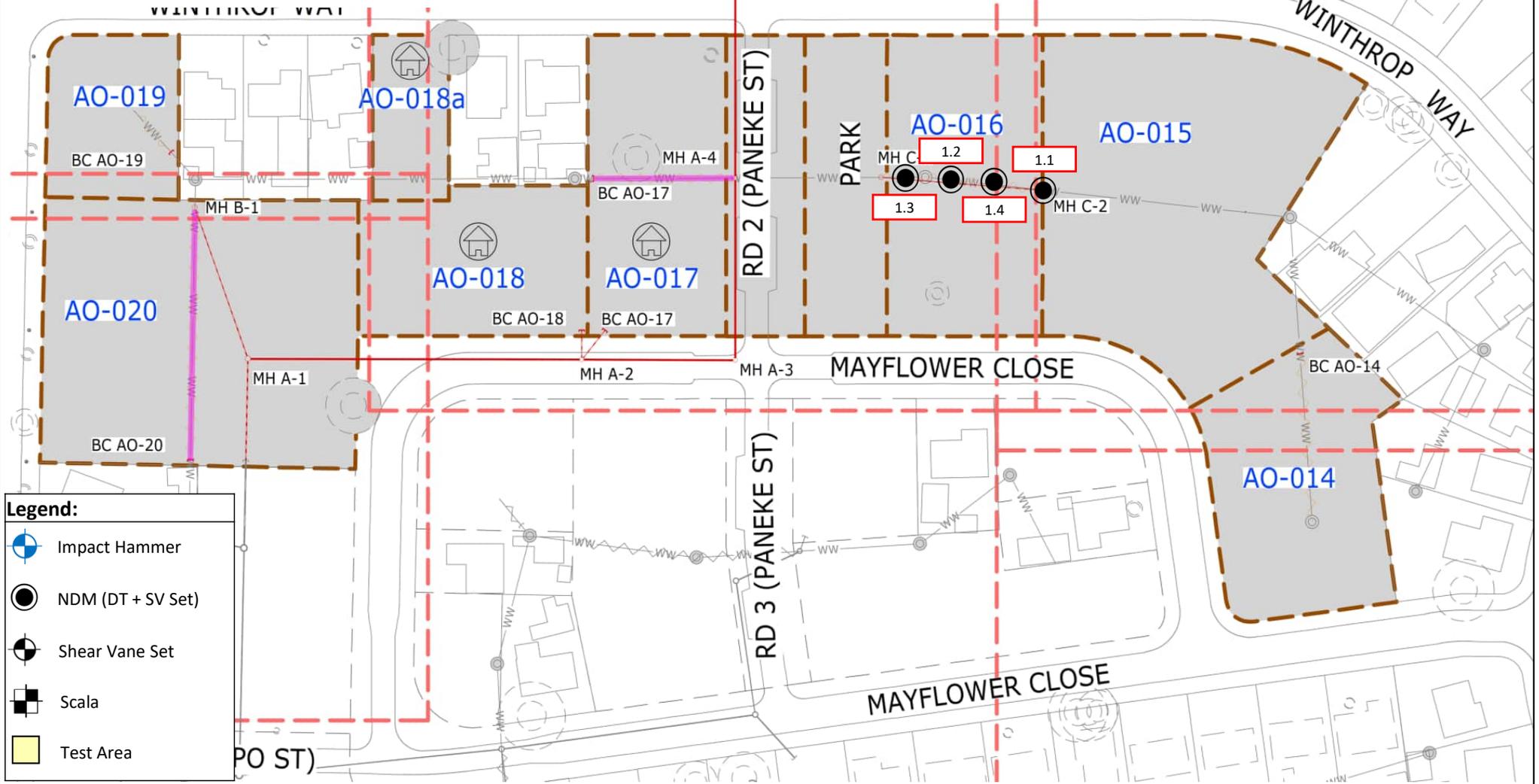
URN	Tech	Date	Location	Layer	Material Type	Test Type	Probe Depth (mm)	NDM 0°			NDM 90°			AVERAGE NDM			Solid Density (t/m³) Assumed	Oven Moisture content (%)	Final Corrected		Shear Vane Reading (kPa)					Retest URN	PASS / FAIL	Comments
								Wet Density (t/m³)	Moisture Content (%)	Air Voids (%)	Wet Density (t/m³)	Moisture Content (%)	Air Voids (%)	Wet Density (t/m³)	Moisture Content (%)	Air Voids (%)			Oven Dry Density (t/m³)	Average Air Voids (%)	Reading 1	Reading 2	Reading 3	Reading 4	Average SV (4 x Tests)			
1.1	KELF	25/01/2023	MH C-1 - MH C-2	Layer 1 400 - 500 mm	ClaysILT	NDM / SV	200	1.81	36.5	2.4	1.81	32.7	4.6	1.81	34.6	3.5	2.70	42.6	1.27	0.0	>235	UTP	>235	>235	>235	-	P	Specification provided by Piritahi requires an average shear vane reading ≥ 130 kPa, with no individual reading < 120 kPa. Air voids have a maximum average < 8%, with no individual reading > 10%.
1.2				NDM / SV		200	1.87	33.3	1.4	1.87	31.8	2.2	1.87	32.6	1.8	2.70	30.4	1.43	3.3	>235	UTP	UTP	212	>223	-	P		
1.3				NDM / SV		200	1.79	43.1	0.0	1.76	35.9	5.5	1.77	39.5	2.7	2.70	44.3	1.23	0.0	134	134	185	155	152	-	P		
1.4				NDM / SV		200	1.80	37.2	2.4	1.81	37.0	2.3	1.81	37.1	2.3	2.70	43.1	1.26	0.0	151	185	148	155	160	-	P		

PROVISIONAL

These results have not yet passed our entire quality assurance process.
 They should be used with caution and may be subject to change.

PROVISIONAL

These results have not yet passed our entire quality assurance process.
They should be used with caution and may be subject to change.



Test Location Plan

Site:	Mangere Precinct TOC45	Job Name:	TOC45-Mangere-Aorere-Stage 2-DIM086/CIM086	Drawn:	KELF	Date:	25/01/2023
Location:	Superlot AO-016 MH C-1 – MH C-2	Job No.:	1041000.0045.1.0/Rep	URN:	1	Date:	25/01/2023
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1



GEOTECHNICS

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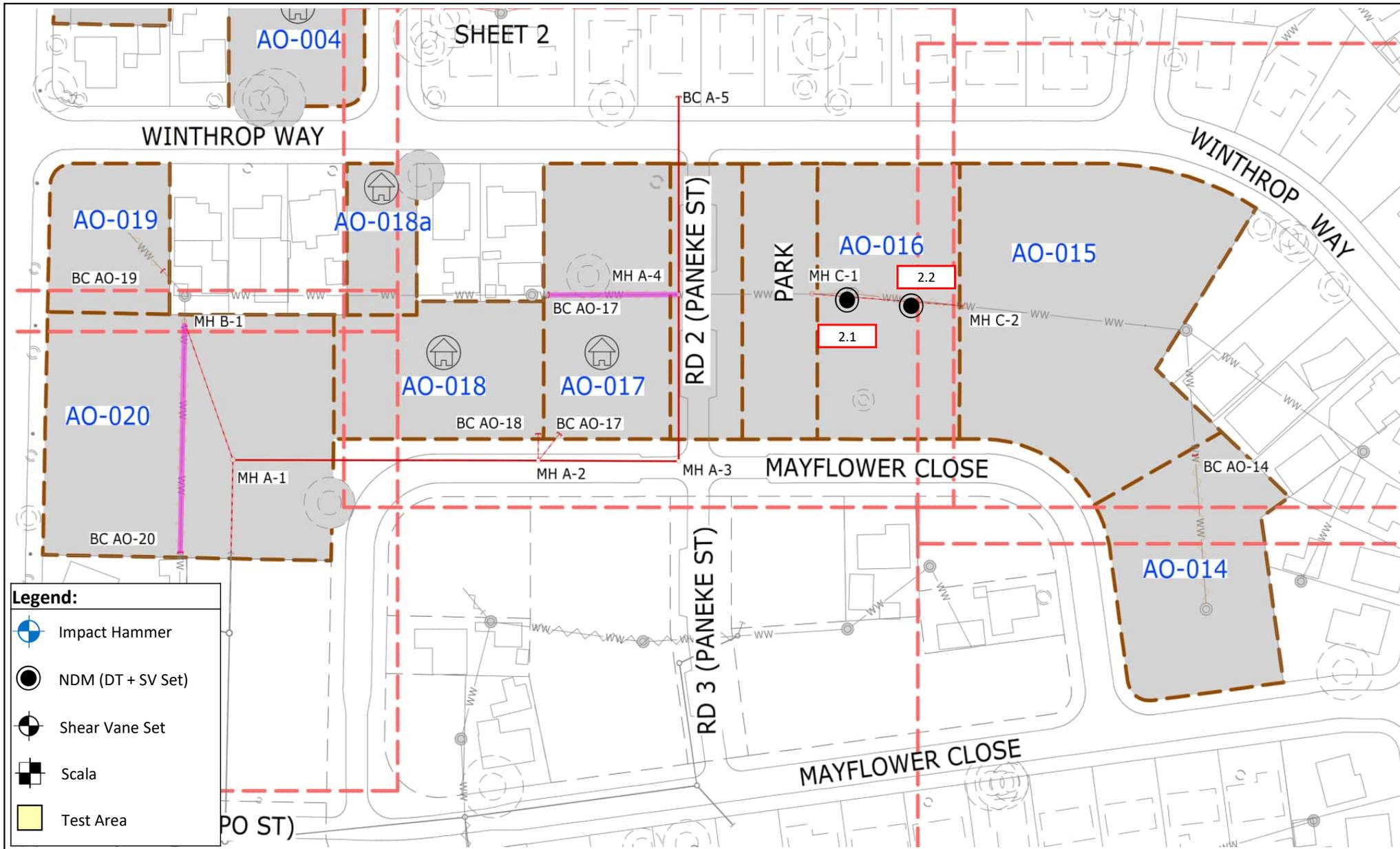
Project Name: Mangere Precinct TOC45
Customer: Piritahi

Project Number	1041000.0045.1.0/
Entered By	KELF/IBSI
Checked By	
Approved by KTP	

URN	Tech	Date	Location	Layer	Material Type	Test Type	Probe Depth (mm)	NDM 0°			NDM 90°			AVERAGE NDM			Solid Density (t/m ³) Assumed	Oven Moisture content (%)	Final Corrected		Shear Vane Reading (kPa)					Retest URN	PASS / FAIL	Comments
								Wet Density (t/m ³)	Moisture Content (%)	Air Voids (%)	Wet Density (t/m ³)	Moisture Content (%)	Air Voids (%)	Wet Density (t/m ³)	Moisture Content (%)	Air Voids (%)			Oven Dry Density (t/m ³)	Average Air Voids (%)	Reading 1	Reading 2	Reading 3	Reading 4	Average SV (4 x Tests)			
2.1	IBSI	9/02/2023	MH C-1 - MH C-2	Layer 3 400 mm	ClaySILT	NDM / SV	200	1.81	41.9	0.0	1.80	37.1	2.6	1.81	39.5	0.9	2.70	43.4	1.26	0.0	157	172	172	157	165	-	P	Specification provided by Piritahi requires an average shear vane reading ≥ 130 kPa, with no individual reading < 120 kPa. Air voids have a maximum average < 8%, with no individual reading > 10%.
2.2						NDM / SV	200	1.80	40.1	1.1	1.75	43.7	1.9	1.77	41.9	1.5	2.70	39.6	1.27	2.7	157	157	141	141	149	-	P	

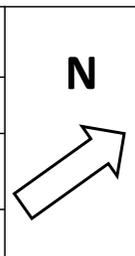
PROVISIONAL

These results have not yet passed our entire quality assurance process.
They should be used with caution and may be subject to change.



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Test Location Plan				PROVISIONAL			
Site:	Mangere Precinct TOC45	Job Name:	TOC45-Mangere-Aorere-Stage 2-DIM086/CIM086	Drawn:	IBSI	Date:	09/02/2023
Location:	Superlot AO-016 MH C-1 – MH C-2	Job No.:	1041000.0045.1.0/	URN:	2	Date:	10/02/2023
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1





Onehunga
Auckland 1061
New Zealand

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Project Name: Mangere Precinct TOC45

Customer: Piritahi

Project Number	1041000.0045.1.0/
Entered By	KELF/IBSI
Checked By	
Approved by KTP	

URN	Tech	Date	Location	Layer	Material Type	Test Type	Probe Depth (mm)	NDM 0°			NDM 90°			AVERAGE NDM			Solid Density (t/m ³) Assumed	Oven Moisture content (%)	Final Corrected		Shear Vane Reading (kPa)					Retest URN	PASS / FAIL	Comments		
								Wet Density (t/m ³)	Moisture Content (%)	Air Voids (%)	Wet Density (t/m ³)	Moisture Content (%)	Air Voids (%)	Wet Density (t/m ³)	Moisture Content (%)	Air Voids (%)			Oven Dry Density (t/m ³)	Average Air Voids (%)	Reading 1	Reading 2	Reading 3	Reading 4	Average SV (4 x Tests)				(P) Pass	(F) Fail
3.1	KELF	10/02/2023	MH C-1 - MH C-2	Layer 4 400 - 500 mm	ClaySILT	NDM / SV	200	1.82	43.4	0.0	1.81	36.1	2.5	1.82	39.8	0.0	2.70	41.4	1.29	0.0	131	134	188	138	148	-	P	Specification provided by Piritahi requires an average shear vane reading ≥ 130 kPa, with no individual reading < 120 kPa. Air voids have a maximum average < 8%, with no individual reading > 10%.		
3.2				NDM / SV		200	1.84	35.3	1.7	1.80	35.7	3.5	1.82	35.5	2.6	2.70	39.7	1.30	0.1	143	176	228	176	181	-	P				
3.3				NDM / SV		200	1.81	36.1	2.9	1.81	35.0	3.6	1.81	35.6	3.2	2.70	32.9	1.36	4.9	195	148	168	181	173	-	P				
3.4				NDM / SV		200	1.85	35.6	0.7	1.86	36.4	0.0	1.86	36.0	0.3	2.70	37.8	1.35	0.0	185	141	134	141	150	-	P				

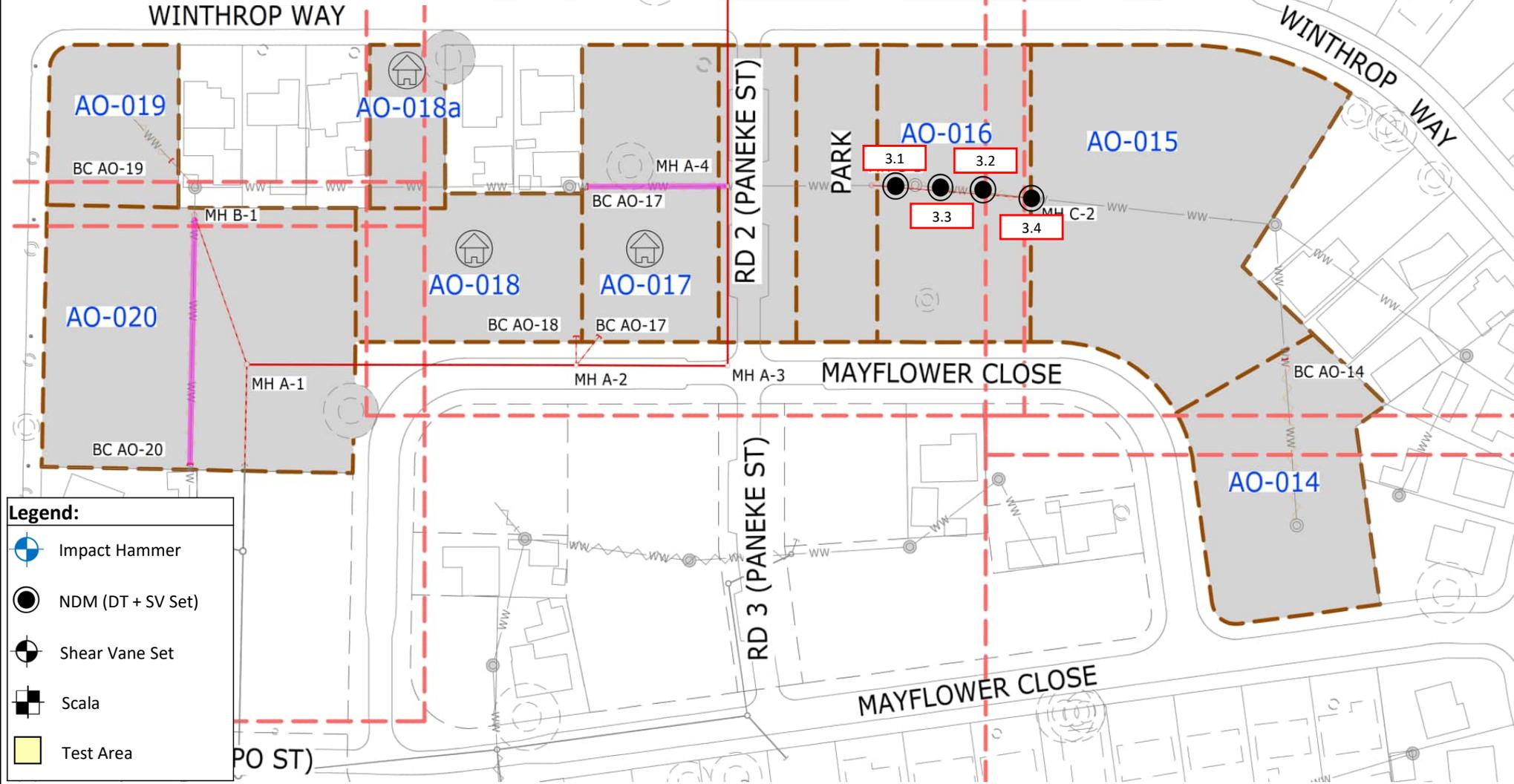
PROVISIONAL

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PROVISIONAL

SHEET 2

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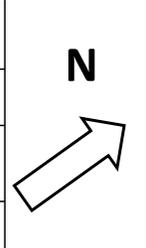


Legend:

- Impact Hammer
- NDM (DT + SV Set)
- Shear Vane Set
- Scala
- Test Area

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Test Location Plan							
Site:	Mangere Precinct TOC45	Job Name:	TOC045-Mangere-Aorere-Stage 2-DIM086/CIM086	Drawn:	KELF	Date:	10/02/2023
Location:	Superlot AO-016 MH C-1 – MH C-2	Job No.:	1041000.0045.1.0/Rep	URN:	3	Date:	10/02/2023
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1





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Project Name : Mangere TOC45 Superlot AO-014-AO-016

Customer : Piritahi

Project Number	1041000.0045.1.0/
Entered By	PEFE/LIWO
Checked By	-
Approved By	-

URN.	Tech.	Date	Location	Layer	Material Type	Test Type	NDM 0°			NDM 90°			AVERAGE NDM			Solid Density (t/m³) Assumed	Oven Moisture content (%)	Final Corrected		Shear Vane Reading (kPa)					Retest URN	PASS / FAIL	Comments
							Wet Density (t/m³)	Moisture Content (%)	Air Voids (%)	Wet Density (t/m³)	Moisture Content (%)	Air Voids (%)	Wet Density (t/m³)	Moisture Content (%)	Air Voids (%)			Oven Dry Density (t/m³)	Average Air Voids (%)	Reading 1	Reading 2	Reading 3	Reading 4	Average SV (4 x Tests)			
1.1	LIWO	3/05/2022	Lateral Line 1	Initial Lift (~500mm)	ClaySILT w/ gravels	NDM / SV	1.80	38.4	1.7	1.80	37.8	2.1	1.80	38.1	1.9	2.70	34.8	1.34	3.9	198	198	198	198	198	-	P	Specification provided by Piritahi requires an average shear vane reading ≥ 130 kPa, with no individual reading < 120 kPa. Air voids have a maximum average < 8%, with no individual reading > 10%.
1.2			Lateral Line 2			NDM / SV	1.78	38.5	2.7	1.79	37.9	3.0	1.78	38.2	2.9	2.70	36.4	1.31	3.9	198	198	195	167	190	-	P	
1.3			Lateral Line 3			NDM / SV	1.81	40.4	0.1	1.80	38.6	1.7	1.81	39.5	0.9	2.70	37.4	1.31	2.1	198	198	184	198	195	-	P	
1.4			Lateral Line 4			NDM / SV	1.70	45.0	3.8	1.70	40.0	6.3	1.70	42.5	5.0	2.70	39.2	1.22	6.8	198	198	198	170	191	-	P	
1.5			Lateral Line 5			NDM / SV	1.71	41.3	5.3	1.70	40.7	6.3	1.70	41.0	5.8	2.70	37.4	1.24	7.8	198	198	198	198	198	-	P	
1.6			Lateral Line 6			NDM / SV	1.78	36.4	3.9	1.79	37.1	3.4	1.79	36.8	3.7	2.70	36.7	1.31	3.7	198	170	184	198	188	-	P	
2.1	PEFE	5/05/2022	Drainage backfill	Initial Lift (~500mm)	claySILT	NDM / SV	1.73	39.7	4.8	1.71	41.4	5.2	1.72	40.6	5.0	2.70	39.9	1.23	5.3	171	163	206	240	195	-	P	
2.2						NDM / SV	1.66	49.3	3.8	1.68	49.5	2.8	1.67	49.4	3.3	2.70	40.9	1.19	7.6	189	141	240	209	195	-	P	
2.3						NDM / SV	1.72	42.2	4.0	1.73	38.8	5.8	1.72	40.5	4.9	2.70	36.5	1.26	7.1	240	240	240	240	240	-	P	
2.4						NDM / SV	1.72	42.7	3.9	1.72	39.7	5.4	1.72	41.2	4.6	2.70	41.1	1.22	4.7	240	240	240	240	240	-	P	
2.5						NDM / SV	1.76	47.8	0.0	1.77	46.7	0.0	1.76	47.3	0.0	2.70	36.1	1.29	5.4	240	240	240	240	240	-	P	
2.6						NDM / SV	1.72	41.1	4.9	1.70	42.2	5.0	1.71	41.7	5.0	2.70	38.2	1.24	6.9	158	146	168	154	156	-	P	
2.7						NDM / SV	1.76	38.4	4.3	1.73	40.0	4.6	1.74	39.2	4.5	2.70	34.1	1.30	7.5	240	240	240	240	240	-	P	
2.8						NDM / SV	1.79	36.1	3.9	1.79	35.5	4.3	1.79	35.8	4.1	2.70	36.3	1.31	3.8	240	240	240	240	240	-	P	
2.9						NDM / SV	1.85	35.9	0.7	1.86	35.6	0.2	1.86	35.8	0.4	2.70	34.8	1.38	1.0	240	240	240	240	240	-	P	
2.10						NDM / SV	1.76	43.1	1.2	1.77	37.6	4.2	1.77	40.4	2.7	2.70	33.3	1.32	6.8	189	149	231	240	202	-	P	

PROVISIONAL

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Legend:

-  Impact Hammer
-  NDM (DT & SV Set)
-  Shear Vane
-  Scala
-  Hand Auger
-  Testing Area

Test Location Plan

Site:	Mangere Precinct TOC45	Job Name:	Mangere Precinct TOC45	Drawn:	LIWO	Date:	3/05/2022
Location:	Superlots AO-014-AO-016	Job No.:	1041000.0029.0.0/1	URN:	1	Date:	3/05/2022
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1



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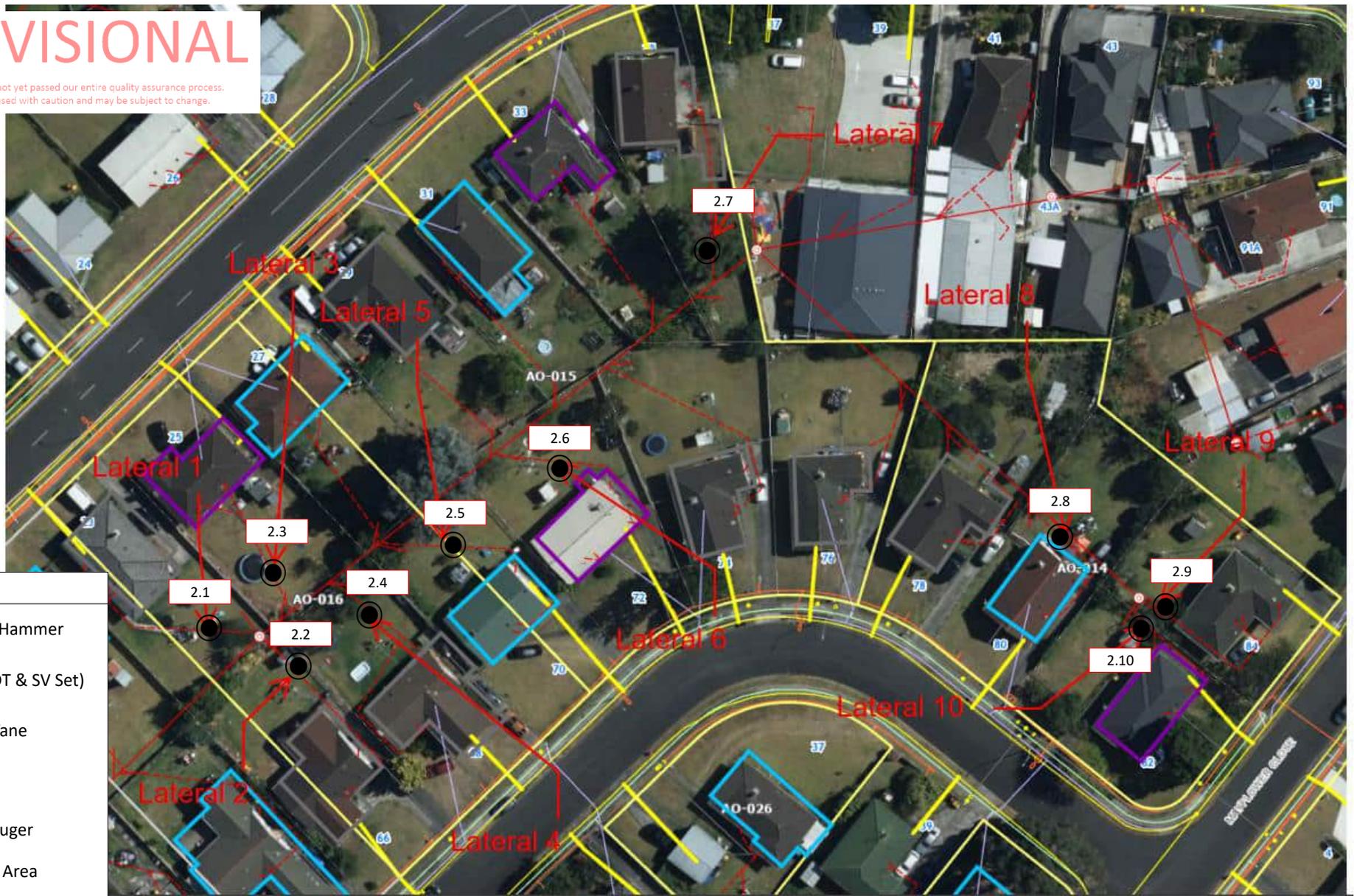
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PROVISIONAL

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Legend:

-  Impact Hammer
-  NDM (DT & SV Set)
-  Shear Vane
-  Scala
-  Hand Auger
-  Testing Area

Test Location Plan

Site:	Mangere Precinct TOC45	Job Name:	Mangere Precinct TOC45	Drawn:	PEFE	Date:	5/05/2022
Location:	Superlots AO-014-AO-016	Job No.:	1041000.0029.0.0/1	URN:	2	Date:	5/05/2022
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1



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Appendix E – Piritahi ITPs

INSPECTION AND TEST PLAN



TOC No.:		ITP No.:	AAAA-DS-ITP-0001 (Revision 2)	EPA No.:	
TOC Name:		ITP Title:	ITP for Earthworks - Cut	Scope of this ITP:	

NOTE: This ITP is to be signed off as one of superlot handover/bulk earthworks construction work pack closeout supporting QA documents at the completion of each superlot/construction work pack.

Item No:	Inspection / Test Description	Hold(H), Witness(W), Review(R), Inspection(I)			Frequency	Test by Whom	Conformance Criteria	Reference Detail	Required Records	Compliance Verification		
		Constructor	CPS/Designer	Territory Authority						Constructor (Y/N/NA)	CPS (Y/N/NA)	Comments
0	Pre-construction Meeting	H	R	R	Prior to commencing the construction works	NA	Construction monitoring, surface surveys, testing frequency and expectations agreed and meeting minuted.	Auckland QAM	Meeting Minutes			
	Subcontractor Kick-off Meeting	H			Once prior to commencing the construction works	NA	Piritahi QA expectations and requirements addressed and meeting minuted	Specified	Meeting Minutes			
1	Cut/Excavation Materials											
1.1	Excavation Material Testing	H	H		Prior to cut	IANZ Laboratory	Material to be excavated shall be tested for contamination if exporting off superlot and/or for use as site-won fill.	Site Specific Remediation Plans	Land Remediation Notice; IANZ Test Report(s); Piritahi Construction Checklist (AAAA-CN-CHK-0008 - Earthworks - Cut)			
2	Construction											
2.1	Original Ground Level Survey and Setting Out	I	R		Prior to clearance and topsoil stripping	NA	Constructor and Designer may jointly visually inspect undertake check or take spot levels as they consider necessary to confirm the accuracy of these drawings and levels.	ATCOP / ACCOP	Piritahi Construction Checklist (AAAA-CN-CHK-0008 - Earthworks - Cut)			
2.2	Clearance and Topsoil Stripping	I	I		After clearance and topsoil stripping	NA	Topsoil (that does not require remediation) shall be removed within the limits of the earthworks and stockpiled within superlot boundaries clear of any 'contaminated' insitu or stockpiled material.	Site Specific Remediation Plans	Land Remediation Notice; Piritahi Construction Checklist (AAAA-CN-CHK-0008 - Earthworks - Cut)			
2.3	Post Topsoil Strip Survey and Setting Out	I	R		Prior to cut	NA	As per drawings	ATCOP / ACCOP	Piritahi Construction Checklist (AAAA-CN-CHK-0008 - Earthworks - Cut)			
2.4	Unsuitable Material Cut to Waste	I	R		As required	NA	Material identified as requiring remediation to be removed as instructed by CPS/Land Remediation Specialists.	Site Specific Remediation Plans	Land Remediation Notice; Piritahi Construction Checklist (AAAA-CN-CHK-0008 - Earthworks - Cut)			
2.5	Suitable Material Cut to Fill	I	R		Prior to excavation for use on site	IANZ Laboratory	Site won material that does not require remediation can be used within the superlot boundary. MDD and OMC are required for site-won fill material. If the site won material is to be exported, then environmental testing is required.	Site Specific Remediation Plans	Land Remediation Notice; IANZ Test Report(s); Piritahi Construction Checklist (AAAA-CN-CHK-0008 - Earthworks - Cut)			
2.6	Cut Subgrade Testing	R	R		As specified by Designer/Geotechnical Team at excavated subgrade level	Constructor/IANZ Laboratory	Specified by Designer/Geotechnical Team prior to commencement of construction (Subject to agreement with Kāinga Ora)	Specified	Piritahi Construction Checklist (AAAA-CN-CHK-0008 - Earthworks - Cut)			
2.7	Undercut & Backfilling Inspection and Testing	H	H		Before and after undercutting and when required (subject to CPS/Designer's instruction)	Constructor/IANZ Laboratory	Specified by CPS/Designer	Specified	Backfill compaction test report(s) as per AAAA-DS-ITP - Earthworks - Fill (if applicable) Piritahi Construction Checklist (AAAA-CN-CHK-0008 - Earthworks - Cut)			
2.8	As-built Survey	R	R		After cutting or backfilling of undercutting	NA	The top of embankment shall not vary more than 0.5m into the flatter area; Section areas (flatter than 1.5:1): levels shall be to within 150mm of the contours or spot levels indicated.	ATCOP / ACCOP	As-built Survey			
2.9	Stabilisation of Final Surfaces	I	I		At finished levels	NA	Smooth drum rolled surfaces with cross fall. Protected with mulch or temporary hardfill cover	Specified	Piritahi Construction Checklist (AAAA-CN-CHK-0008 - Earthworks - Cut)			
3	Post Construction											
3.1	Final Inspection (Piritahi Internal)	H	H		At the completion of cut (i.e., each superlot and/or construction work pack)	NA	No unacceptable defects	ACTOP Section 16.9.1	Photos			

Constructor	Name:	Signature:	Date:
CPS	Name:	Signature:	Date:
QA	Name:	Signature:	Date:

INSPECTION AND TEST PLAN



TOC No.:		ITP No.:	AAAA-DS-ITP-0002 (Revision 2)	EPA No.:	
TOC Name:		ITP Title:	ITP for Earthworks - Fill (Including decanting Earth Bunds and Backfilled Redundant Service Trenches)	Scope of this ITP:	

NOTE: This ITP is to be signed off as one of superlot handover/bulk earthworks construction work pack closeout supporting QA documents at the completion of each superlot/construction work pack.

Item No.	Inspection / Test Description	Hold(H), Witness(W), Review(R), Inspection(I)			Frequency	Test by Whom	Conformance Criteria	Reference Detail	Required Records	Compliance Verification		
		Constructor	CPS/Designer	Territory Authority						Constructor (Y/N/NA)	CPS (Y/N/NA)	Comments
0	Pre-construction Meeting	H	R	R	Prior to commencing the construction works	NA	Construction monitoring expectations, frequency and timeframes agreed between construction and testing teams and meeting minutes distributed	NA	Meeting Minutes			
	Subcontractor Kick-off Meeting	H			Once prior to commencing the construction works	NA	Piritahi QA expectations and requirements addressed and meeting minutes	Specified	Meeting Minutes			
1	Fill Materials											
1.1	Fill Materials	H	R		Prior to filling 1 initial test for each material and then 1 test per 5,000 m3 for that particular material type.	IANZ Laboratory	Prior to filling (site stockpile to be sampled); (1) Site won material: MDD and OMC (standard Proctor compaction test); (2) Imported cohesive fill: Natural Water Content, Atterberg Limits and Linear Shrinkage for NZS3604 expansivity, and Shrink Swell Test for AS2870; (3) Imported granular: PSD, MDD, OMC and Weathering Quality Index (NZS4407:2015 Test 3.11)	Specified	IANZ Test Report(s); Piritahi Construction Checklist (AAAA-CN-CHK-0009 - Earthworks - Fill)			
2	Construction QA/QC Requirements											
2.1	Setting Out	I	I		Prior to undercutting/filling	NA	By Construction Surveyor as specified in Civil Specification	ACCOP ATCOP	Piritahi Construction Checklist (AAAA-CN-CHK-0009 - Earthworks - Fill)			
2.2	Inspection of subgrade or exposed undercut surface	H	H		Prior to filling or backfilling of undercutting	NA	By CPS and Geotechnical Engineer/Geologist prior to filling	Specified	Piritahi Construction Checklist (AAAA-CN-CHK-0009 - Earthworks - Fill)			
2.3	Testing of subgrade	R	R		Prior to filling: - 10 m by 10 m grid for bulk earthworks; or, - 1 test (Shear Vane&Scala) per 15m for trenches	Constructor	a) Shear Vane: >=60 kPa and b) Scala Penetrometer Test: >=1 blow per 50 mm to 1 m below base of fill	Specified	Shear Vane & Scala Test Records; Piritahi Construction Checklist (AAAA-CN-CHK-0009 - Earthworks - Fill)			
2.4	Testing of undercut surface (if applicable)	R	R		Prior to backfilling of undercutting	Constructor	Specified by CPS/Designer	Specified	Shear Vane/Scala Test Records when required; Piritahi Construction Checklist (AAAA-CN-CHK-0009 - Earthworks - Fill)			
2.5	Benching	I	I		As directed by CPS/Designer	NA	Any portion of the ground whose slope is steeper than three horizontal to one vertical shall be benched before filling is placed on it; The base of the benches shall be sloped inwards at a slope of 12 horizontal to 1 vertical. The longitudinal profile of each bench shall be graded to ensure adequate drainage and safe discharge of water.	Specified	Piritahi Construction Checklist (AAAA-CN-CHK-0009 - Earthworks - Fill)			
2.6	Inspection of Earthwork Filling in Layers	I	I		During filling at every 0.5m height intervals	NA	The thickness of each loose layer (lift) is to be approximately 250mm to 300mm achieve maximum 200mm thick compacted layers	Specified	Piritahi Construction Checklist (AAAA-CN-CHK-0009 - Earthworks - Fill)			
2.7	Compaction Testing in Layers	H	R		Testing undertaken at every 0.5 m height interval. Within Road Reserves: - Clegg 10m by 10m grid and NDM 20m by 20m grid for bulk earthworks; or, - 1 test (Clegg&NDM) per 15m for trenches Within Superlot Boundaries: 1. Cohesive fill (imported or site won): - Shear vane 10m by 10m grid and NDM 20m by 20m grid for bulk earthworks; or, - 1 test (Shear vane&NDM) per 15m for trenches 2. Granular fill (imported): - Clegg 10m by 10m grid and NDM 20m by 20m grid for bulk earthworks; or, - 1 test (Clegg&NDM) per 15m for trenches 3. SPR/ROP: - Shear vane and Clegg 10m by 10m grid and NDM 20m by 20m grid for bulk earthworks; or, - 1 test (Shear vane, Clegg&NDM) per 15m for trenches NOTE: NDM not required for less than 600mm deep backfilled trenches within Superlots, however, Clegg or shear vane tests are required.	Constructor - Shear vane & Clegg; IANZ Laboratory - NDM	Within Road Reserves: GAP65: CIV (average) >= 30 and no single value < 28 and MDD > =95%; GAP40: CIV (average) >= 23 and no single value < 21 and MDD >= 95% Other approved materials: to be checked with CPS/Designer. Within Superlot Boundaries 1. Cohesive fill (imported or site won): - Shear vane (average over 4 readings) >= 130kPa and no single reading < 120kPa; and, - NDM with Air Voids (average) < 8% and no single Air Void > 10% 2. Granular fill (imported): - GAP65: CIV (average) >= 30 and no single value < 28 and MDD >= 92% - GAP40: CIV (average) >= 23 and no single value < 21 and MDD >= 92% - GAP100: CIV (average) >= 30 and no single value < 28 and MDD >= 92% 3. SPR/ROP: - CIV (average) >= 30 and no single value < 28; and, - Shear vane (average over 4 readings) >= 130kPa and no single reading < 120kPa; and, - MDD >= 92% and Air Voids (Average) < 8% and no single Air Void > 10%	Specified	IANZ NDM Test Report(s); Shear Vane and/or Clegg Test Records; Piritahi Construction Checklist (AAAA-CN-CHK-0009 - Earthworks - Fill)			
2.8	Tolerances (As-built)	H	R		As specified	NA	Final Road Subgrades: -20 +0mm Fill; Batters: -0 +150mm; All other fill areas: +0 -75mm	ATCOP Section 16.9.1	As-built Survey			
2.9	Stabilisation of final surfaces	I	I		At finished levels	NA	Smooth drum rolled surfaces with cross fall. Protected with mulch or temporary hardfill cover	Specified	Piritahi Construction Checklist (AAAA-CN-CHK-0009 - Earthworks - Fill)			
3	Post Construction											
3.1	Final Inspection (Piritahi Internal)	H	H		At the completion of fill (i.e., each superlot and/or construction work pack)	NA	No unacceptable defects	ACTOP Section 16.9.1	Photos			

Constructor	Name:	Signature:	Date:
CPS	Name:	Signature:	Date:
QA	Name:	Signature:	Date:

INSPECTION AND TEST PLAN - STORMWATER

TOC No.:		ITP No.:	AAAA-DS-ITP-0005 (Revision 6)	EPA No.:	
TOC Name:		ITP Title:	ITP for Stormwater	Scope of this ITP:	

NOTE: This ITP is to be signed off as one of CS3/CS4/CoA and/or EACC application supporting QA documents at the completion of each EPA.

Item No.	Inspection / Test Description	Hold(H), Witness(W), Review(R), Inspection(I)			Frequency	Test by Whom	Conformance Criteria	Reference Detail	Required Records	Compliance Verification		
		Constructor	CPS/Designer	Territory Authority						Constructor (Y/N/NA)	CPS (Y/N/NA)	Comments
0	Pre-construction Meeting	H	R	R	Prior to commencing the construction works	NA	Construction monitoring level agreed and meeting minuted	NA	Meeting Minutes			
	Subcontractor Kick-off Meeting	H			Once prior to commencing the construction works	NA	Piritahi QA expectations and requirements addressed and meeting minuted	Specified	Meeting Minutes			
1	Material Compliance											
1.1	Pipes and Miscellaneous Precast Units (Manhole, Lid, Catchpit/Cesspit, Inlet and Outlet, etc)	I/R	R		Material Certificate/Compliance Statement to cover all the pipes & other precast units is to be provided to Quality Engineer for each Precinct every 6 months or at a change of source.	NA	1. Types, sizes and classes are as per the design drawings. 2. Conformance criteria refer to relevant material standards.	NA	Material Certificate(s)/Compliance Statement(s) from Supplier			
1.2	Aggregates for Bedding, Haunch, Overlay and Backfill	I/R	R		Material Certificate/Compliance Statement is to be provided to Quality Engineer for each Precinct every 6 months or at a change of source.	IANZ Laboratory	1. The granular material for bedding, haunch, overlay and backfill shall be hard clean, chemically stable crushed stone that would not break down when wetted. Shale or gravely conglomerates are not suitable materials. 2. The granular materials agreed to use include GAP7, GAP20, GAP40, GAP65 and ROP/SPR. 3. Supplier shall provide IANZ Test Reports for PSD, MDD and DI (Density Index). 4. Aggregate MDD and DI test results will be accepted as they are. 5. Aggregate PSD shall meet the specified grading curve envelopes as per Appendix A of this ITP (To be provided).	Specified	PSD, MDD and DI IANZ Test Report(s) from Supplier			
1.3	In situ Concrete	I/R	R		When products are delivered to site and before products are incorporated into the works	NA	In situ concrete for all drainage works shall be a minimum of 20MPa unless specified otherwise on the drawings.	Specified	Concrete Delivery Dockets			
1.4	Steel Reinforcement & Miscellaneous Steel Inc. Bolts and Nuts	I/R	R		When products are delivered to site and before products are incorporated into the works	NA	1. Types, sizes and classes are as per the design drawings. 2. Conformance criteria refer to relevant material standards.	NA	Material Certificate(s)/Compliance Statement(s) from Supplier			
2	Construction QA/QC Requirements											
2.1	Pipeline											
2.1.1	Trenching	I			After excavation	NA	1. Trench location, width level and depth as per drawing and setting out. 2. The minimum width of the trench should be such that the barrel of the pipe is not closer than 150mm to the trench wall or shoring. 3. Enough space for the trench shields should be allowed for.	Specified	Piritahi Construction Checklist for Stormwater			

INSPECTION AND TEST PLAN - STORMWATER

2.1.2	Trench Foundation & Undercutting	H	H	<p>Shear Vane for trench foundation to be tested: - For trenches $\geq 30m$ - every 15m - For trenches $< 30m$ - min 2 tests</p> <p>Clegg for backfilling of undercut to be tested: - For trenches $\geq 30m$ - 1 test every 15m - For trenches $< 30m$ - min 2 tests</p>	Constructor	<p>1. Remove any unsuitable foundation material in accordance with the below table:</p> <table border="1"> <thead> <tr> <th>Shear vane strength (in clay/silt)</th> <th>Scala Penetrometer (in sand/gravel)</th> <th>Undercut and backfill</th> </tr> </thead> <tbody> <tr> <td>≥ 80 kPa</td> <td>≤ 50mm per blow</td> <td>No undercut required</td> </tr> <tr> <td>60-80 kPa</td> <td>60mm per blow</td> <td>200mm compacted hardfill</td> </tr> <tr> <td>40-60 kPa</td> <td>80mm per blow</td> <td>350mm compacted hardfill</td> </tr> <tr> <td>20-40 kPa</td> <td>100mm per blow</td> <td>500mm compacted hardfill</td> </tr> <tr> <td>< 20 kPa or highly organic</td> <td>200mm per blow</td> <td>Contact the design engineer</td> </tr> </tbody> </table> <p>2. Undercutting to have a transition slope of 1:5 between the undercut section and the normal foundation.</p> <p>3. Backfill material (GAP65 or GAP40) shall be selected as per the design drawings and the required compactness is as below:</p> <p>For backfill (1) GAP 65: CIV (average over 30m length/average per manhole) > 30 & no single value < 28. OR: (2) GAP 40: CIV (average over 30m length/ average per manhole) > 23 & no single value < 21.</p> <p>Note: If it is not possible to undertake the required Clegg tests, then a visual inspection – Hold Point shall be done by CPS Lead who may engage the geotechnical engineer when required.</p>	Shear vane strength (in clay/silt)	Scala Penetrometer (in sand/gravel)	Undercut and backfill	≥ 80 kPa	≤ 50 mm per blow	No undercut required	60-80 kPa	60mm per blow	200mm compacted hardfill	40-60 kPa	80mm per blow	350mm compacted hardfill	20-40 kPa	100mm per blow	500mm compacted hardfill	< 20 kPa or highly organic	200mm per blow	Contact the design engineer	Specified	Piritahi Construction Checklist for Stormwater; Shear Vane/Clegg Test Record			
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< 20 kPa or highly organic	200mm per blow	Contact the design engineer																											
2.1.3	Pipe Bedding	I/H	R	<p>Clegg for bedding to be tested: - For trenches $\geq 30m$ - 1 test every 15m - For trenches $< 30m$ - min 2 tests</p> <p>Note: Every Clegg test shall have 4 readings recorded.</p>	Constructor	<p>H2 Support Type Bedding Depth: 100mm if $\phi < 1500$mm 150mm if $\phi > 1500$mm Max layer thickness 150mm</p> <p>Bedding material (GAP7 or GAP20) shall be selected as per the design drawings and the required compactness is as below:</p> <p>- GAP 7 & GAP 20: CIV (average over 30m length/ average per manhole) > 12 & CIV no less than 10.</p> <p>Note: If it is not possible to undertake the required Clegg tests, then a visual inspection – Hold Point shall be done by CPS Lead who may engage the geotechnical engineer when required.</p>	Auckland Council SWCoP Drawing SW03 NZS3725 Table 5																						
2.1.4	Pipe Haunch	I/H	R	<p>Clegg for haunch to be tested: - For trenches $\geq 30m$ - 1 test every 15m per layer - For trenches $< 30m$ - min 2 tests per layer</p>	Constructor	<p>H2 Support Type Bedding Depth: Haunch Zone Depth = $0.3 \times$ Pipe ϕ Max layer thickness 150mm</p> <p>Haunch material (GAP7 or GAP20) shall be selected as per the design drawings and the required compactness is as below:</p> <p>- GAP 7 & GAP 20: CIV (average over 30m length/ average per manhole) > 12 & CIV no less than 10.</p> <p>Note: If it is not possible to undertake the required Clegg tests, then a visual inspection – Hold Point shall be done by CPS Lead who may engage the geotechnical engineer when required.</p>	Auckland Council SWCoP Drawing SW03 NZS3725 Table 5	Piritahi Construction Checklist for Stormwater; Clegg Test Record																					
2.1.5	Pipe laying	H	H	After pipe laying	NA	<p>1. Pipe laying should be true to the line, levels and grades as per design drawings. 2. Pipe position tolerance: ± 50mm; pipe level tolerance: ± 30mm 3. Horizontal/Position tolerance for HDD, pipe jacking, boring or tunnelling: ± 100mm</p> <p>Note: AC SW CoP for Grade: As-built gradients shall be no less than 0.1% and no greater than 25%.</p>	Specified	Piritahi Construction Checklist for Stormwater																					

INSPECTION AND TEST PLAN - STORMWATER

2.1.6	Trench Backfill	I/R	R		<p>Within Road Corridor - Clegg to be tested (No need NDM):</p> <ul style="list-style-type: none"> - For trenches in Berms every 15m min two per layer - For trenches in Carriageways and footpaths every 5m min two tests per layer <p>Within Superlot Boundary - Clegg to be tested:</p> <ul style="list-style-type: none"> - For trenches ≥30m of more - every 15m per layer - For trenches <30m - min 2 tests per layer 	Constructor	<p>Within Road Reserves Including Berms (Up to Subbase Level) Hardfill Backfill placed in layers not exceeding 200mm thickness & Compacted to: Carriageway CIV > 25 which is equivalent to 90% MDD - Under the Sub-Base Footpath CIV > 15 - Under the Sub-Base Beam CIV > 10</p> <p>Within Superlot Boundaries</p> <ol style="list-style-type: none"> Granular fill shall be free of organics (max particle size 150mm) and placed in layers not exceeding 200mm. Overlay material (GAP65, GAP40 or GAP20) shall be selected as per the design drawings and the required compactness is as below: <p>(1) GAP 65: CIV (average over 30m length/average per manhole) > 30 & no single value < 28. OR: (2) GAP 40: CIV (average over 30m length/ average per manhole) > 23 & no single value < 21. (3) GAP 20: CIV (average over 30m length/ average per manhole) > 12 & CIV no less than 10.</p>	National Code of Practice Section 5.5.3, 5.5.4 and 5.5.5	Piritahi Construction Checklist for Stormwater; Clegg Test Record																					
2.2	Manhole																													
2.2.1	Manhole Excavation	I			After excavation	NA	As per drawing and setting out (location, width level and depth)	NA	Piritahi Construction Checklist for Stormwater																					
2.2.2	Manhole Foundation & Undercutting	H	H		<p>Shear Vane for manhole foundation to be tested:</p> <ul style="list-style-type: none"> - Minimum 2 tests per manhole location <p>Clegg for backfilling of undercut to be tested:</p> <ul style="list-style-type: none"> - Minimum 2 tests per manhole location 	Constructor	<ol style="list-style-type: none"> Remove any unsuitable foundation material in accordance with the below table: <table border="1"> <thead> <tr> <th>Shear vane strength (in clay/silt)</th> <th>Scala Penetrometer (in sand/gravel)</th> <th>Undercut and backfill</th> </tr> </thead> <tbody> <tr> <td>≥80 kPa</td> <td>≤50mm per blow</td> <td>No undercut required</td> </tr> <tr> <td>60-80 kPa</td> <td>60mm per blow</td> <td>200mm compacted hardfill</td> </tr> <tr> <td>40-60 kPa</td> <td>80mm per blow</td> <td>350mm compacted hardfill</td> </tr> <tr> <td>20-40 kPa</td> <td>100mm per blow</td> <td>500mm compacted hardfill</td> </tr> <tr> <td><20 kPa or highly organic</td> <td>200mm per blow</td> <td>Contact the design engineer</td> </tr> </tbody> </table> <ol style="list-style-type: none"> Undercutting to have a transition slope of 1:5 between the undercut section and the normal manhole or connecting pipeline foundation. Backfill material (GAP65 or GAP40) shall be selected as per the design drawings and the required compactness is as below: <p>(1) GAP 65: CIV (average over 30m length/average per manhole) > 30 & no single value < 28. OR: (2) GAP 40: CIV (average over 30m length/ average per manhole) > 23 & no single value < 21.</p> <p>Note: If it is not possible to undertake the required Clegg tests, then a visual inspection – Hold Point shall be done by CPS Lead who may engage the geotechnical engineer when required.</p>	Shear vane strength (in clay/silt)	Scala Penetrometer (in sand/gravel)	Undercut and backfill	≥80 kPa	≤50mm per blow	No undercut required	60-80 kPa	60mm per blow	200mm compacted hardfill	40-60 kPa	80mm per blow	350mm compacted hardfill	20-40 kPa	100mm per blow	500mm compacted hardfill	<20 kPa or highly organic	200mm per blow	Contact the design engineer	Specified	Piritahi Construction Checklist for Stormwater; Shear Vane/Clegg Test Record			
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<20 kPa or highly organic	200mm per blow	Contact the design engineer																												
2.2.3	Manhole Bedding	I/H	R		<p>Clegg for bedding to be tested:</p> <ul style="list-style-type: none"> - Minimum 2 tests per manhole location 	Constructor	<p>Bedding Depth and material as per detail drawing (min 75mm)</p> <p>Compaction values: CIV > 12 & CIV no less than 10.</p>	Specified	Construction Checklist for Stormwater; Clegg Test Record																					
2.2.4	Manhole Installation & As-builts	I/R	I		After installation and before backfilling	Constructor	<ol style="list-style-type: none"> Manhole installation shall be as per the drawings with the top levelling with the surrounding surfacing and benching inside the manhole (which typically is after backfilling of manhole). Manhole position tolerance: +/-50mm Manhole lid level tolerance: +/-30mm 	Specified	Piritahi Construction Checklist for Stormwater																					

INSPECTION AND TEST PLAN - STORMWATER

2.2.5	Manhole Backfilling	I/R	R		<p style="color: red;">Within Road Reserves Including Berms - Compaction for backfilling to be tested using Clegg (No need NDM): - Minimum 2 tests per manhole location</p> <p style="color: red;">Within Superlot Boundaries - Compaction for backfilling to be tested using Clegg (aggregates) or Share Vane (cohesive fill) - No need NDM: - Minimum 2 tests per manhole location</p>	Constructor	<p style="color: red;">Within Road Reserves Including Berms (Up to Subbase Level) Hardfill Backfill placed in layers not exceeding 200mm thickness & compacted to CIV>25 which is equivalent to 90% MDD.</p> <p>Within Superlot Boundaries 1. Cohesive or granular fill shall be free of organics (max particle size 150mm) placed in layers not exceeding 300mm. 2. Lower backfill zone material (Cohesive, GAP65, GAP40, GAP20 or ROP/SPR) shall be selected as per the design drawings and the required compactness is as below: (1) Cohesive fill: Shear Vane (Average over 4 readings) >130 and no single reading < 120. OR: (2) GAP 65: CIV (average over 30m length/average per manhole) > 30 & no single value < 28. OR: (3) GAP 40: CIV (average over 30m length/ average per manhole) > 23 & no single value < 21. (4) GAP 20: CIV (average over 30m length/ average per manhole) > 12 & CIV no less than 10. (5) ROP/SPR: (To be provided by Designer)</p>	Specified	Piritahi Construction Checklist for Stormwater; Shear Vane/Clegg Test Record			
2.3	CCTV Inspection	I/R	H		After backfilling and/or before road surfacing as per Piritahi CCTV Process	NA	Meet Auckland Council relevant standards	SWCoP	CCTV Tapes and Logs			
2.4	As-built Survey	R	R		As required	NA	Meet Auckland Council relevant standards	SWCoP	As-built Site Checker & Certificated As-built Drawings			
3	Post Construction											
3.1	Final Inspection (Piritahi Internal)	H	H	H	At the completion of each EPA	NA	No unacceptable defects	SWCoP	Photos			

Appendix F – Foundation maintenance and footing performance: a homeowner’s guide

Foundation Maintenance and Footing Performance: A Homeowner's Guide



PUBLISHING
BTF 18-2011
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Information
Sheet 10/91

Buildings can and often do move. This movement can be up, down, lateral or rotational. The fundamental cause of movement in buildings can usually be related to one or more problems in the foundation soil. It is important for the homeowner to identify the soil type in order to ascertain the measures that should be put in place in order to ensure that problems in the foundation soil can be prevented, thus protecting against building movement.

This Building Technology File is designed to identify causes of soil-related building movement, and to suggest methods of prevention of resultant cracking in buildings.

Soil Types

The types of soils usually present under the topsoil in land zoned for residential buildings can be split into two approximate groups – granular and clay. Quite often, foundation soil is a mixture of both types. The general problems associated with soils having granular content are usually caused by erosion. Clay soils are subject to saturation and swell/shrink problems.

Classifications for a given area can generally be obtained by application to the local authority, but these are sometimes unreliable and if there is doubt, a geotechnical report should be commissioned. As most buildings suffering movement problems are founded on clay soils, there is an emphasis on classification of soils according to the amount of swell and shrinkage they experience with variations of water content. The table below is Table 2.1 from AS 2870-2011, the Residential Slab and Footing Code.

Causes of Movement

Settlement due to construction

There are two types of settlement that occur as a result of construction:

- Immediate settlement occurs when a building is first placed on its foundation soil, as a result of compaction of the soil under the weight of the structure. The cohesive quality of clay soil mitigates against this, but granular (particularly sandy) soil is susceptible.
- Consolidation settlement is a feature of clay soil and may take place because of the expulsion of moisture from the soil or because of the soil's lack of resistance to local compressive or shear stresses. This will usually take place during the first few months after construction, but has been known to take many years in exceptional cases.

These problems are the province of the builder and should be taken into consideration as part of the preparation of the site for construction. Building Technology File 19 (BTF 19) deals with these problems.

Erosion

All soils are prone to erosion, but sandy soil is particularly susceptible to being washed away. Even clay with a sand component of say 10% or more can suffer from erosion.

Saturation

This is particularly a problem in clay soils. Saturation creates a bog-like suspension of the soil that causes it to lose virtually all of its bearing capacity. To a lesser degree, sand is affected by saturation because saturated sand may undergo a reduction in volume, particularly imported sand fill for bedding and blinding layers. However, this usually occurs as immediate settlement and should normally be the province of the builder.

Seasonal swelling and shrinkage of soil

All clays react to the presence of water by slowly absorbing it, making the soil increase in volume (see table below). The degree of increase varies considerably between different clays, as does the degree of decrease during the subsequent drying out caused by fair weather periods. Because of the low absorption and expulsion rate, this phenomenon will not usually be noticeable unless there are prolonged rainy or dry periods, usually of weeks or months, depending on the land and soil characteristics.

The swelling of soil creates an upward force on the footings of the building, and shrinkage creates subsidence that takes away the support needed by the footing to retain equilibrium.

Shear failure

This phenomenon occurs when the foundation soil does not have sufficient strength to support the weight of the footing. There are two major post-construction causes:

- Significant load increase.
- Reduction of lateral support of the soil under the footing due to erosion or excavation.

In clay soil, shear failure can be caused by saturation of the soil adjacent to or under the footing.

Tree root growth

Trees and shrubs that are allowed to grow in the vicinity of footings can cause foundation soil movement in two ways:

- Roots that grow under footings may increase in cross-sectional size, exerting upward pressure on footings.
- Roots in the vicinity of footings will absorb much of the moisture in the foundation soil, causing shrinkage or subsidence.

Unevenness of Movement

The types of ground movement described above usually occur unevenly throughout the building's foundation soil. Settlement due to construction tends to be uneven because of:

- Differing compaction of foundation soil prior to construction.
- Differing moisture content of foundation soil prior to construction.

Movement due to non-construction causes is usually more uneven still. Erosion can undermine a footing that traverses the flow or can create the conditions for shear failure by eroding soil adjacent to a footing that runs in the same direction as the flow.

Saturation of clay foundation soil may occur where subfloor walls create a dam that makes water pond. It can also occur wherever there is a source of water near footings in clay soil. This leads to a severe reduction in the strength of the soil which may create local shear failure.

Seasonal swelling and shrinkage of clay soil affects the perimeter of the building first, then gradually spreads to the interior. The swelling process will usually begin at the uphill extreme of the building, or on the weather side where the land is flat. Shrinkage gradually reaches the interior soil as absorption continues. Shrinkage usually begins where the sun's heat is greatest.

Effects of Uneven Soil Movement on Structures

Erosion and saturation

Erosion removes the support from under footings, tending to create subsidence of the part of the structure under which it occurs. Brickwork walls will resist the stress created by this removal of support by bridging the gap or cantilevering until the bricks or the mortar bedding fail. Older masonry has little resistance. Evidence of failure varies according to circumstances and symptoms may include:

- Step cracking in the mortar beds in the body of the wall or above/below openings such as doors or windows.
- Vertical cracking in the bricks (usually but not necessarily in line with the vertical beds or perpend).)

Isolated piers affected by erosion or saturation of foundations will eventually lose contact with the bearers they support and may tilt or fall over. The floors that have lost this support will become bouncy, sometimes rattling ornaments etc.

Seasonal swelling/shrinkage in clay

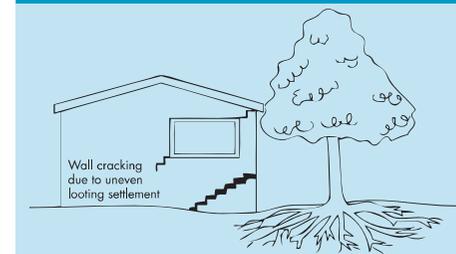
Swelling foundation soil due to rainy periods first lifts the most exposed extremities of the footing system, then the remainder of the perimeter footings while gradually permeating inside the building footprint to lift internal footings. This swelling first tends to create a dish effect, because the external footings are pushed higher than the internal ones.

The first noticeable symptom may be that the floor appears slightly dished. This is often accompanied by some doors binding on the floor or the door head, together with some cracking of cornice mitres. In buildings with timber flooring supported by bearers and joists, the floor can be bouncy. Externally there may be visible dishing of the hip or ridge lines.

As the moisture absorption process completes its journey to the innermost areas of the building, the internal footings will rise. If the spread of moisture is roughly even, it may be that the symptoms will temporarily disappear, but it is more likely that swelling will be uneven, creating a difference rather than a disappearance in symptoms. In buildings with timber flooring supported by bearers and joists, the isolated piers will rise more easily than the strip footings or piers under walls, creating noticeable doming of flooring.

As the weather pattern changes and the soil begins to dry out, the external footings will be first affected, beginning with the locations where the sun's effect is strongest. This has the effect of lowering the

Trees can cause shrinkage and damage



external footings. The doming is accentuated and cracking reduces or disappears where it occurred because of dishing, but other cracks open up. The roof lines may become convex.

Doming and dishing are also affected by weather in other ways. In areas where warm, wet summers and cooler dry winters prevail, water migration tends to be toward the interior and doming will be accentuated, whereas where summers are dry and winters are cold and wet, migration tends to be toward the exterior and the underlying propensity is toward dishing.

Movement caused by tree roots

In general, growing roots will exert an upward pressure on footings, whereas soil subject to drying because of tree or shrub roots will tend to remove support from under footings by inducing shrinkage.

Complications caused by the structure itself

Most forces that the soil causes to be exerted on structures are vertical – i.e. either up or down. However, because these forces are seldom spread evenly around the footings, and because the building resists uneven movement because of its rigidity, forces are exerted from one part of the building to another. The net result of all these forces is usually rotational. This resultant force often complicates the diagnosis because the visible symptoms do not simply reflect the original cause. A common symptom is binding of doors on the vertical member of the frame.

Effects on full masonry structures

Brickwork will resist cracking where it can. It will attempt to span areas that lose support because of subsided foundations or raised points. It is therefore usual to see cracking at weak points, such as openings for windows or doors.

In the event of construction settlement, cracking will usually remain unchanged after the process of settlement has ceased.

With local shear or erosion, cracking will usually continue to develop until the original cause has been remedied, or until the subsidence has completely neutralised the affected portion of footing and the structure has stabilised on other footings that remain effective.

In the case of swell/shrink effects, the brickwork will in some cases return to its original position after completion of a cycle, however it is more likely that the rotational effect will not be exactly reversed, and it is also usual that brickwork will settle in its new position and will resist the forces trying to return it to its original position. This means that in a case where swelling takes place after construction and cracking occurs, the cracking is likely to at least partly remain after the shrink segment of the cycle is complete. Thus, each time the cycle is repeated, the likelihood is that the cracking will become wider until the sections of brickwork become virtually independent.

With repeated cycles, once the cracking is established, if there is no other complication, it is normal for the incidence of cracking to stabilise, as the building has the articulation it needs to cope with the problem. This is by no means always the case, however, and monitoring of cracks in walls and floors should always be treated seriously.

Upheaval caused by growth of tree roots under footings is not a simple vertical shear stress. There is a tendency for the root to also exert lateral forces that attempt to separate sections of brickwork after initial cracking has occurred.

GENERAL DEFINITIONS OF SITE CLASSES

Class	Foundation
A	Most sand and rock sites with little or no ground movement from moisture changes
S	Slightly reactive clay sites, which may experience only slight ground movement from moisture changes
M	Moderately reactive clay or silt sites, which may experience moderate ground movement from moisture changes
H1	Highly reactive clay sites, which may experience high ground movement from moisture changes
H2	Highly reactive clay sites, which may experience very high ground movement from moisture changes
E	Extremely reactive sites, which may experience extreme ground movement from moisture changes

Notes

1. Where controlled fill has been used, the site may be classified A to E according to the type of fill used.
2. Filled sites. Class P is used for sites which include soft fills, such as clay or silt or loose sands; landslide; mine subsidence; collapsing soils; soil subject to erosion; reactive sites subject to abnormal moisture conditions or sites which cannot be classified otherwise.
3. Where deep-seated moisture changes exist on sites at depths of 3 m or greater, further classification is needed for Classes M to E (M-D, H1-D, H2-D and E-D).

The normal structural arrangement is that the inner leaf of brickwork in the external walls and at least some of the internal walls (depending on the roof type) comprise the load-bearing structure on which any upper floors, ceilings and the roof are supported. In these cases, it is internally visible cracking that should be the main focus of attention, however there are a few examples of dwellings whose external leaf of masonry plays some supporting role, so this should be checked if there is any doubt. In any case, externally visible cracking is important as a guide to stresses on the structure generally, and it should also be remembered that the external walls must be capable of supporting themselves.

Effects on framed structures

Timber or steel framed buildings are less likely to exhibit cracking due to swell/shrink than masonry buildings because of their flexibility. Also, the doming/dishing effects tend to be lower because of the lighter weight of walls. The main risks to framed buildings are encountered because of the isolated pier footings used under walls. Where erosion or saturation causes a footing to fall away, this can double the span which a wall must bridge. This additional stress can create cracking in wall linings, particularly where there is a weak point in the structure caused by a door or window opening. It is, however, unlikely that framed structures will be so stressed as to suffer serious damage without first exhibiting some or all of the above symptoms for a considerable period. The same warning period should apply in the case of upheaval. It should be noted, however, that where framed buildings are supported by strip footings there is only one leaf of brickwork and therefore the externally visible walls are the supporting structure for the building. In this case, the subfloor masonry walls can be expected to behave as full brickwork walls.

Effects on brick veneer structures

Because the load-bearing structure of a brick veneer building is the frame that makes up the interior leaf of the external walls plus perhaps the internal walls, depending on the type of roof, the building can be expected to behave as a framed structure, except that the external masonry will behave in a similar way to the external leaf of a full masonry structure.

Water Service and Drainage

Where a water service pipe, a sewer or stormwater drainage pipe is in the vicinity of a building, a water leak can cause erosion, swelling or saturation of susceptible soil. Even a minuscule leak can be enough to saturate a clay foundation. A leaking tap near a building can have the same effect. In addition, trenches containing pipes can become watercourses even though backfilled, particularly where broken rubble is used as fill. Water that runs along these trenches can be responsible for serious erosion, interstrata seepage into subfloor areas and saturation.

Pipe leakage and trench water flows also encourage tree and shrub roots to the source of water, complicating and exacerbating the problem. Poor roof plumbing can result in large volumes of rainwater being concentrated in a small area of soil:

- Incorrect falls in roof guttering may result in overflows, as may gutters blocked with leaves etc.

- Corroded guttering or downpipes can spill water to ground.
- Downpipes not positively connected to a proper stormwater collection system will direct a concentration of water to soil that is directly adjacent to footings, sometimes causing large-scale problems such as erosion, saturation and migration of water under the building.

Seriousness of Cracking

In general, most cracking found in masonry walls is a cosmetic nuisance only and can be kept in repair or even ignored. The table below is a reproduction of Table C1 of AS 2870-2011.

AS 2870-2011 also publishes figures relating to cracking in concrete floors, however because wall cracking will usually reach the critical point significantly earlier than cracking in slabs, this table is not reproduced here.

Prevention/Cure

Plumbing

Where building movement is caused by water service, roof plumbing, sewer or stormwater failure, the remedy is to repair the problem. It is prudent, however, to consider also rerouting pipes away from the building where possible, and relocating taps to positions where any leakage will not direct water to the building vicinity. Even where gully traps are present, there is sometimes sufficient spill to create erosion or saturation, particularly in modern installations using smaller diameter PVC fixtures. Indeed, some gully traps are not situated directly under the taps that are installed to charge them, with the result that water from the tap may enter the backfilled trench that houses the sewer piping. If the trench has been poorly backfilled, the water will either pond or flow along the bottom of the trench. As these trenches usually run alongside the footings and can be at a similar depth, it is not hard to see how any water that is thus directed into a trench can easily affect the foundation's ability to support footings or even gain entry to the subfloor area.

Ground drainage

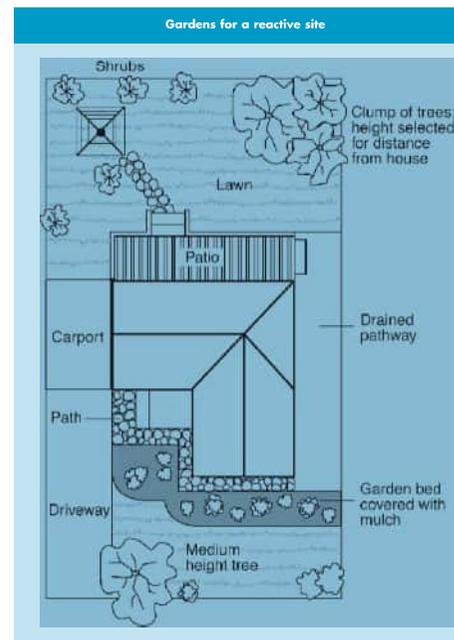
In all soils there is the capacity for water to travel on the surface and below it. Surface water flows can be established by inspection during and after heavy or prolonged rain. If necessary, a grated drain system connected to the stormwater collection system is usually an easy solution.

It is, however, sometimes necessary when attempting to prevent water migration that testing be carried out to establish water table height and subsoil water flows. This subject is referred to in BTF 19 and may properly be regarded as an area for an expert consultant.

Protection of the building perimeter

It is essential to remember that the soil that affects footings extends well beyond the actual building line. Watering of garden plants, shrubs and trees causes some of the most serious water problems.

For this reason, particularly where problems exist or are likely to occur, it is recommended that an apron of paving be installed around as much of the building perimeter as necessary. This paving should



extend outwards a minimum of 900 mm (more in highly reactive soil) and should have a minimum fall away from the building of 1:60. The finished paving should be no less than 100 mm below brick vent bases.

It is prudent to relocate drainage pipes away from this paving, if possible, to avoid complications from future leakage. If this is not practical, earthenware pipes should be replaced by PVC and backfilling should be of the same soil type as the surrounding soil and compacted to the same density.

Except in areas where freezing of water is an issue, it is wise to remove taps in the building area and relocate them well away from the building – preferably not uphill from it (see BTF 19).

It may be desirable to install a grated drain at the outside edge of the paving on the uphill side of the building. If subsoil drainage is needed this can be installed under the surface drain.

Condensation

In buildings with a subfloor void such as where bearers and joists support flooring, insufficient ventilation creates ideal conditions for condensation, particularly where there is little clearance between the floor and the ground. Condensation adds to the moisture already present in the subfloor and significantly slows the process of drying out. Installation of an adequate subfloor ventilation system, either natural or mechanical, is desirable.

Warning: Although this Building Technology File deals with cracking in buildings, it should be said that subfloor moisture can result in the development of other problems, notably:

- Water that is transmitted into masonry, metal or timber building elements causes damage and/or decay to those elements.
- High subfloor humidity and moisture content create an ideal environment for various pests, including termites and spiders.
- Where high moisture levels are transmitted to the flooring and walls, an increase in the dust mite count can ensue within the living areas. Dust mites, as well as dampness in general, can be a health hazard to inhabitants, particularly those who are abnormally susceptible to respiratory ailments.

The garden

The ideal vegetation layout is to have lawn or plants that require only light watering immediately adjacent to the drainage or paving edge, then more demanding plants, shrubs and trees spread out in that order.

Overwatering due to misuse of automatic watering systems is a common cause of saturation and water migration under footings. If it is necessary to use these systems, it is important to remove garden beds to a completely safe distance from buildings.

Existing trees

Where a tree is causing a problem of soil drying or there is the existence or threat of upheaval of footings, if the offending roots are subsidiary and their removal will not significantly damage the tree, they should be severed and a concrete or metal barrier placed vertically in the soil to prevent future root growth in the direction of the building. If it is not possible to remove the relevant roots without damage to the tree, an application to remove the tree should be made to the local authority. A prudent plan is to transplant likely offenders before they become a problem.

Information on trees, plants and shrubs

State departments overseeing agriculture can give information regarding root patterns, volume of water needed and safe distance from buildings of most species. Botanic gardens are also sources of information. For information on plant roots and drains, see Building Technology File 17.

Excavation

Excavation around footings must be properly engineered. Soil supporting footings can only be safely excavated at an angle that allows the soil under the footing to remain stable. This angle is called the angle of repose (or friction) and varies significantly between soil types and conditions. Removal of soil within the angle of repose will cause subsidence.

Remediation

Where erosion has occurred that has washed away soil adjacent to footings, soil of the same classification should be introduced and compacted to the same density. Where footings have been undermined, augmentation or other specialist work may be required. Remediation of footings and foundations is generally the realm of a specialist consultant.

Where isolated footings rise and fall because of swell/shrink effect, the homeowner may be tempted to alleviate floor bounce by filling the gap that has appeared between the bearer and the pier with blocking. The danger here is that when the next swell segment of the cycle occurs, the extra blocking will push the floor up into an accentuated dome and may also cause local shear failure in the soil. If it is necessary to use blocking, it should be by a pair of fine wedges and monitoring should be carried out fortnightly.

This BTF was prepared by John Lewer FAIB, MIAMA, Partner, Construction Diagnosis.

CLASSIFICATION OF DAMAGE WITH REFERENCE TO WALLS

Description of typical damage and required repair	Approximate crack width limit (see Note 3)	Damage category
Hairline cracks	<0.1 mm	0
Fine cracks which do not need repair	<1 mm	1
Cracks noticeable but easily filled. Doors and windows stick slightly.	<5 mm	2
Cracks can be repaired and possibly a small amount of wall will need to be replaced. Doors and windows stick. Service pipes can fracture. Weathertightness often impaired.	5–15 mm (or a number of cracks 3 mm or more in one group)	3
Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Window and door frames distort. Walls lean or bulge noticeably, some loss of bearing in beams. Service pipes disrupted.	15–25 mm but also depends on number of cracks	4

The information in this and other issues in the series was derived from various sources and was believed to be correct when published.

The information is advisory. It is provided in good faith and not claimed to be an exhaustive treatment of the relevant subject.

Further professional advice needs to be obtained before taking any action based on the information provided.

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